

Cynthia Vodopivec Electric Energy, Inc. Luminant 6555 Sierra Dr. Irving, TX 75039

September 29, 2020

Sent via email

Mr. Andrew R. Wheeler, EPA Administrator Environmental Protection Agency 1200 Pennsylvania Avenue, N.W. Mail Code 5304-P Washington, DC 20460

Re: Joppa Power Station Alternative Closure Demonstration

Dear Administrator Wheeler:

Electric Energy, Inc. (Electric Energy) hereby submits this request to the U.S. Environmental Protection Agency (EPA) for approval of a site-specific alternative deadline to initiate closure pursuant to 40 C.F.R. § 257.103(f)(2) for the East Ash Pond located at the Joppa Power Station near Joppa, Illinois. Electric Energy is requesting an extension pursuant to 40 C.F.R. § 257.103(f)(2) so that the East Ash Pond may continue to receive CCR and non-CCR wastestreams after April 11, 2021, and complete closure no later than October 17, 2028.

Enclosed is a demonstration prepared by Burns & McDonnell that addresses all of the criteria in 40 C.F.R. § 257.103(f)(2)(i)-(iv) and contains the documentation required by 40 C.F.R. § 257.103(f)(2)(v). As allowed by the agency, in lieu of hard copies of these documents, electronic files were submitted to Kirsten Hillyer, Frank Behan, and Richard Huggins via email. If you have any questions regarding this submittal, please contact Phil Morris at 618-343-7794 or phil.morris@vistracorp.com.

Sincerely,

Cynthin E. Wdg

Cynthia Vodopivec VP - Environmental Health & Safety

Enclosure

cc: Kirsten Hillyer Frank Behan Richard Huggins





CCR Surface Impoundment Demonstration for a Site-Specific Alternative to Initiation of Closure Deadline



Electric Energy, Inc.

Joppa Power Station Project No. 122702

> Revision 0 9/28/2020



CCR Surface Impoundment Demonstration for a Site-Specific Alternative to Initiation of Closure Deadline

prepared for

Electric Energy, Inc. Joppa Power Station Joppa, Illinois

Project No. 122702

Revision 0 9/28/2020

prepared by

Burns & McDonnell Engineering Company, Inc. Kansas City, Missouri

INDEX AND CERTIFICATION

Electric Energy, Inc. CCR Surface Impoundment Demonstration for a Site-Specific Alternative to Initiation of Closure Deadline Project No. 122702

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Certification

I hereby certify, as a Professional Engineer in the state of Illinois, that the information in this document as noted in the above Report Index was assembled under my direct personal charge. This report is not intended or represented to be suitable for reuse by the Electric Energy, Inc. or others without specific verification or adaptation by the Engineer.



Edward T. Tohill, P.E., (Illinois License No. 062-056915)

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LIST OF ABBREVIATIONS

Abbreviation	Term/Phrase/Name	
CCR	Coal Combustion Residual	
CFR	Code of Federal Regulations	
Electric Energy	Electric Energy, Inc.	
ELG Rule	Effluent Limitations Guidelines and Standards for the Steam Electric Power Generating Point Source Category	
EPA	Environmental Protection Agency	
Joppa	Joppa Power Station	
RCRA	Resource Conservation and Recovery Act	
SWPPP	Stormwater Pollution Prevention Plan	

1.0 EXECUTIVE SUMMARY

Electric Energy, Inc. (Electric Energy) submits this request to the U.S. Environmental Protection Agency (EPA) for approval of a site-specific alternative deadline to initiate closure pursuant to 40 C.F.R. § 257.103(f)(2) — "Permanent Cessation of a Coal-Fired Boiler(s) by a Date Certain"— for the East Ash Pond located at the Joppa Power Station (Joppa) in Illinois. The East Ash Pond is a 111-acre CCR surface impoundment used to manage CCR and non-CCR wastestreams at Joppa. As discussed herein, the boilers at the station will retire and the impoundment will complete closure no later than October 17, 2028. Therefore, Electric Energy is requesting an extension pursuant to 40 C.F.R. § 257.103(f)(2) so that the East Ash Pond may continue to receive CCR and non-CCR waste streams after April 11, 2021, and complete closure no later than October 17, 2028.

2.0 INTRODUCTION

Joppa is an 802-megawatt coal-fueled electric generating station near Joppa, Illinois, that utilizes the 111acre East Ash Pond to manage sluiced bottom ash, economizer ash, non-marketable dry fly ash (when not hauled offsite for beneficial use), dredged material from the settling lagoon and cooling water intake, and non-CCR wastewaters. The northern portion of the impoundment was constructed in 1973 and the southern portion between the years of 1977 and 1985. The various non-CCR wastewaters routed to the East Ash Pond originate from the water treatment floor drain, demineralizer regeneration flows, reverse osmosis reject, and stormwater. A site plan is provided on Figure 1 in Appendix A, and the plant water balance diagram is included in Appendix B. Note that the East Ash Pond is referred to as the Ash Pond on the water balance diagram.

On April 17, 2015, the Environmental Protection Agency ("EPA) issued the federal Coal Combustion Residual (CCR) Rule, 40 C.F.R. Part 257, Subpart D, to regulate the disposal of CCR materials generated at coal-fueled units. The rule is being administered under Subtitle D of the Resource Conservation and Recovery Act (RCRA, 42 U.S.C. § 6901 et seq.). On August 28, 2020, the EPA Administrator issued revisions to the CCR Rule that require all unlined surface impoundments to initiate closure by April 11, 2021, unless an alternative deadline is requested and approved. 40 C.F.R. § 257.101(a)(1) (85 Fed. Reg. 53,516 (Aug. 28, 2020)). Specifically, owners and operators of a CCR surface impoundment may continue to receive CCR and non-CCR wastestreams if the facility will cease operation of the coal-fired boiler(s) and complete closure of the impoundments within certain specified timeframes. 40 C.F.R. § 257.103(f)(2). To qualify for an alternative closure deadline under § 257.103(f)(2), a facility must meet the following four criteria:

- 1. § 257.103(f)(2)(i) No alternative disposal capacity is available on-site or off-site. An increase in costs or the inconvenience of existing capacity is not sufficient to support qualification.
- 2. § 257.103(f)(2)(ii) Potential risks to human health and the environment from the continued operation of the CCR surface impoundment have been adequately mitigated;
- 3. § 257.103(f)(2)(iii) The facility is in compliance with the CCR rule, including the requirement to conduct any necessary corrective action; and
- 4. § 257.103(f)(2)(iv) The coal-fired boilers must cease operation and closure of the impoundment must be completed within the following timeframes:
 - a. For a CCR surface impoundment that is 40 acres or smaller, the coal-fired boiler(s) must cease operation and the CCR surface impoundment must complete closure no later than October 17, 2023.

b. For a CCR surface impoundment that is larger than 40 acres, the coal-fired boiler(s) must cease operation, and the CCR surface impoundment must complete closure no later than October 17, 2028.

Section 257.103(f)(2)(v) sets out the documentation that must be provided to EPA to demonstrate that the four criteria set out above have been met. Therefore, this demonstration is organized based on the documentation requirements of §§ 257.103(f)(2)(v)(A) - (D).

3.0 DOCUMENTATION OF NO ALTERNATIVE DISPOSAL CAPACITY

To demonstrate that the criteria in § 257.103(f)(2)(i) has been met, the following provides documentation that no alternative disposal capacity is currently available on-site or off-site for each CCR and non-CCR wastestream that IPRG seeks to continue placing into the Ash Pond after April 11, 2021. Consistent with the regulations, neither an increase in costs nor the inconvenience of existing capacity was used to support qualification under this criteria. Instead, as EPA explained in the preamble to the proposed Part A revisions, "it would be illogical to require [] facilities [ceasing power generation] to construct new capacity to manage CCR and non-CCR wastestreams." 84 Fed. Reg. 65,941, 65,956 (Dec. 2, 2019). EPA again reiterated in the preamble to the final revisions that "[i]n contrast to the provision under § 257.103(f)(1), the owner or operator does not need to develop alternative capacity because of the impending closure of the coal fired boiler. Since the coal-fired boiler will shortly cease power generation, it would be illogical to require these facilities to construct new capacity to manage CCR and non-CCR wastestreams." 85 Fed. Reg. at 53,547. Thus, new construction or the development of new alternative disposal capacity was not considered a viable option for any wastestream discussed below.

3.1 Site-Layout and Wastewater Processes

The East Ash Pond receives all CCR sluice flows and a portion of the non-CCR wastewater flows onsite. Many of the remaining plant process flows are routed through the Settling Lagoon for treatment, as shown in Appendix B. The Settling Lagoon is not authorized to receive the CCR sluice flows and is not large enough to independently treat the total volume of the plant process water flows. Electric Energy constructed a new off-site CCR landfill in 2009 to receive scrubber by-products; however, the landfill was never made operational and is unable to receive sluiced materials. The landfill is currently unusable due to the deterioration of the landfill cell freeze protection layer and damage to the leachate collection system and cell perimeter berms. Additionally, since the landfill has never been operated, a landfill operator, leachate hauling contractor, and leachate disposal facility have not been retained.

3.2 CCR Wastestreams

Electric Energy evaluated each CCR wastestream placed in the East Ash Pond at Joppa. For the reasons discussed below in Table 3-1, each of the following CCR wastestreams must continue to be placed in the East Ash Pond at Joppa due to lack of alternative capacity both on and off-site.

CCR Wastestreams	Average Flow (MGD)	Alternative Capacity Currently Available? YES/NO	Details
Bottom Ash, Economizer Ash, and non-CCR mill rejects Sluice	0.1	NO	There is no potential alternative for on or off-site disposal of this wet-generated CCR wastestream.
			The fly ash is collected dry and is currently conditioned and disposed in the East Ash Pond intermittently when not hauled offsite for beneficial use. Approximately 95% of the fly ash is beneficially reused off-site.
			The remaining conditioned fly ash is placed in the East Ash Pond, which will facilitate pond closure in the near future. This beneficial reuse of the fly ash will be reflected in the pond closure plan.
Dry Fly Ash (includes air heater ash)	NA (Dry)	NO	Electric Energy does not have a CCR landfill or another CCR surface impoundment located onsite that is available or ready to accept this material. Consequently, there are currently no on- site alternatives for this wastestream.
			As discussed above, the CCR landfill constructed for Joppa in 2009 is inactive and would require significant repairs and/or improvements prior to receiving CCR material. Other offsite landfills are over 40 miles away from the site and Electric Energy does not have a contract with any of these landfills for this material.

Table 3-1: Joppa CCR Wastestreams

For the bottom ash sluice flow, there is no currently available onsite infrastructure to support dry handling of bottom ash or elimination of this wastestream. As stated previously, since Electric Energy has elected to pursue the option to permanently cease the use of the coal fired boilers by a date certain, developing alternative disposal capacity is "illogical," to use EPA's words, and also counterproductive to the work to retire the boilers and close the impoundments. As long as Electric Energy continues to wet handle the bottom ash, economizer ash, and mill reject materials, there are no other onsite CCR impoundments to receive and treat these flows and it is not feasible to dispose of the wet-handled material offsite. As EPA explained in the preamble of the 2015 rule, it is not possible for sites that sluice CCR material to an impoundment to eliminate the impoundment and dispose of the material offsite. *See* 80 Fed. Reg. 21,301, 21,423 (Apr. 17, 2015) ("[W]hile it is possible to transport dry ash off-site to [an] alternate disposal facility

that is simply not feasible for wet-generated CCR. Nor can facilities immediately convert to dry handling systems."). As a result, the conditions at Joppa satisfy the demonstration requirement in § 257.103(f)(2)(i).

Any non-marketable dry fly ash must also be placed in the Joppa East Ash Pond due to lack of existing alternative capacity both on and off-site. Significant modifications would be required to dispose of the non-marketable dry fly ash in the Joppa inactive landfill, including modifications to current plant operations and improvements to the protective cover and the leachate collection and handling system at the landfill. Moreover, since Electric Energy has elected to pursue the option to permanently cease the use of the coal fired boilers by a date certain, developing this alternative disposal capacity would be "illogical," to use EPA's words, and also counterproductive to the work to retire the boilers and close the impoundments. Modifications to current plant operations would also be required to use other offsite landfills that are over 40 miles away; and having to transport the material such a distance to an offsite landfill would present safety concerns. Accordingly, the non-marketable fly ash must be placed in the only available onsite disposal location (i.e., the East Ash Pond) when not hauled offsite for beneficial use due to seasonal market impacts. Consequently, in order to continue to operate and generate electricity, Joppa must continue to use the 111-acre CCR surface impoundment to manage the CCR wastestreams discussed above.

3.3 Non-CCR Wastestreams

Electric Energy evaluated each non-CCR wastestream placed in the East Ash Pond at Joppa. For the reasons discussed below in Table 3-2, each of the following non-CCR wastestreams must continue to be placed in the East Ash Pond at Joppa due to lack of alternative capacity both on and off-site.

Non-CCR Wastestreams	Average Flow (MGD)	Alternative Capacity Currently Available? YES/NO	Details
Settling Lagoon and Cooling Water Intake Dredged Material	Intermittent	NO	The settling lagoon and cooling water intake require dredging to ensure the capacity of the settling lagoon and cooling water flow is maintained. The dredged material is then placed in the East Ash Pond. This stream requires significant retention time for TSS removal to meet the permitted discharge limits. A new treatment system and permit modifications would be required to reroute to a new or existing permitted outfall. There is no on- site alternative for this wastestream.

 Table 3-2: Joppa Non-CCR Wastestreams

Non-CCR Wastestreams	Average Flow (MGD)	Alternative Capacity Currently Available? YES/NO	Details
Water Treatment Building Floor Drains (including wash waters and demineralizer regeneration flows	0.01	NO	Permit modifications would be required, and additional piping would need to be installed to reroute to a new or existing permitted outfall
Reverse Osmosis Reject	0.1	YES	As required by the NPDES permit, this wastestream is currently piped to both the East Ash Pond and settling lagoon. The settling lagoon will serve as the alternative disposal capacity for this wastestream.
Ash Landfill Leachate	Intermittent	NO	As allowed by the NPDES permit, this wastestream would be trucked to the East Ash Pond for disposal. This flow will not occur unless the landfill modifications are completed, and the facility is placed in service.

The reverse osmosis reject wastewater can currently be discharged to the settling lagoon, as allowed by the NPDES permit. The settling lagoon serves as current and practical alternative disposal capacity for this wastestream; therefore, it is not a part of this request.

There is potential to discharge a portion of the remaining non-CCR flows to alternate locations; however, this would require permit modifications and installation of new piping and potentially a new treatment system including non-CCR ponds, clarifiers, and/or storage tank(s). As stated previously, since Electric Energy has elected to pursue the option to permanently cease the use of the coal fired boilers by a certain date, developing alternative disposal capacity is "illogical," to use EPA's words, and also counterproductive to the work to retire the boilers and close the impoundments. There is currently no existing installed infrastructure at the plant to support reroute of these flows (except the reverse osmosis reject). For the reasons discussed above, each of the remaining non-CCR wastestreams must continue to be placed in the East Ash Pond due to lack of alternative capacity both on and off-site. Consequently, in order to continue to operate and generate electricity, Joppa must continue to use the 111-acre East Ash Pond to manage the non-CCR wastestreams discussed above.

4.0 **RISK MITIGATION PLAN**

To demonstrate that the criteria in § 257.103(f)(2)(ii) has been met, Electric Energy has prepared and attached a Risk Mitigation Plan for the Joppa East Ash Pond (see Attachment 1).

5.0 DOCUMENTATION AND CERTIFICATION OF COMPLIANCE

In the Part A rule preamble, EPA reiterates that compliance with the CCR rule is a prerequisite to qualifying for an alternative closure extension, as it "provides some guarantee that the risks at the facility are properly managed and adequately mitigated." 85 Fed. Reg. at 53,543. EPA further stated that it "must be able to affirmatively conclude that facility meets this criterion prior to any continued operation." 85 Fed. Reg. at 53,543. Accordingly, EPA "will review a facility's current compliance with the requirements governing groundwater monitoring systems." 85 Fed. Reg. at 53,543. In addition, EPA will also "require and examine a facility's corrective action documentation, structural stability documents and other pertinent compliance information." 85 Fed. Reg. at 53,543. Therefore, EPA is requiring a certification of compliance and specific compliance documentation be submitted as part of the demonstration. 40 C.F.R. § 257.103(f)(2)(v)(C).

To demonstrate that the criteria in § 257.103(f)(2)(iii) has been met, Electric Energy is submitting the following information as required by § 257.103(f)(2)(v)(C):

5.1 Owner's Certification of Compliance - § 257.103(f)(2)(v)(C)(1)

I hereby certify that, based on my inquiry of those persons who are immediately responsible for compliance with environmental regulations for the East Ash Pond at Joppa, the facility is in compliance with all of the requirements contained in 40 C.F.R. Part 257, Subpart D – Standards for the Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments. The Joppa CCR compliance website is up-to-date and contains all the necessary documentation and notification postings.

On behalf of Electric Energy:

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Cynthia Vodopivec VP - Environmental Health & Safety September 28, 2020

5.2 Visual representation of hydrogeologic information - § 257.103(f)(2)(v)(C)(2)

Consistent with the requirements of § 257.103(f)(2)(v)(C)(2)(i) - (iii), Electric Energy has attached the following items to this demonstration:

- Map(s) of groundwater monitoring well locations in relation to the CCR unit (Attachment 2)
- Well construction diagrams and drilling logs for all groundwater monitoring wells (Attachment 3)
- Maps that characterize the direction of groundwater flow accounting for seasonal variations (Attachment 4)

5.3 Groundwater monitoring results - § 257.103(f)(2)(v)(C)(3)

Tables summarizing constituent concentrations at each groundwater monitoring well through the first 2020 semi-annual monitoring period are included as Attachment 5.

5.4 Description of site hydrogeology including stratigraphic cross-sections - § 257.103(f)(2)(v)(C)(4)

A description of the site hydrogeology and stratigraphic cross-sections of the site are included as Attachment 6.

5.5 Corrective measures assessment - § 257.103(f)(2)(v)(C)(5)

Background sampling began at Joppa in late 2015 and continued for eight consecutive quarters. The first semiannual detection monitoring samples were collected in November 2017. The first assessment monitoring samples were collected in June 2018. The results, through the 2020 monitoring period, indicate that the Joppa East Ash Pond is currently in assessment monitoring, with no exceedances of the Appendix IV parameters. Accordingly, an assessment of corrective measures is not currently required at the site. Joppa will continue to conduct groundwater monitoring in accordance with all state and federal requirements.

5.6 Remedy selection progress report - § 257.103(f)(2)(v)(C)(6)

As noted above, an assessment of corrective measures and the resulting selection of remedy is not currently required for the East Ash Pond.

5.7 Structural stability assessment - § 257.103(f)(2)(v)(C)(7)

Pursuant to § 257.73(d), the initial structural stability assessment for the East Ash Pond was prepared in October 2016 and is included as Attachment 7.

5.8 Safety factor assessment - § 257.103(f)(2)(v)(C)(8)

Pursuant to § 257.73(e), the initial safety factor assessment for the East Ash Pond was prepared in October

2016 and is included as Attachment 8.

6.0 DOCUMENTATION OF CLOSURE COMPLETION TIMEFRAME

To demonstrate that the criteria in § 257.103(f)(2)(iv) has been met, "the owner or operator must submit the closure plan required by § 257.102(b) and a narrative that specifies and justifies the date by which they intend to cease receipt of waste into the unit in order to meet the closure deadlines." An addendum to the closure plan for the East Ash Pond is included as Attachment 9.

In order for a CCR surface impoundment over 40 acres to continue to receive CCR and non-CCR wastestreams after the initial April 11, 2021 deadline, the coal-fired boiler(s) at the facility must cease operation and the CCR surface impoundment must complete closure no later than October 17, 2028. As discussed below, Joppa will begin construction of the East Ash Pond closure by October 17, 2025, and cease placing wastestreams into the East Ash Pond by July 17, 2027, in order for closure to be completed by this deadline.

Table 6-1 is included below to summarize the major tasks and durations associated with closing the East Ash Pond in place. These durations are consistent with the durations experienced in the closure of over 500 acres of other CCR impoundments already completed by Electric Energy and its affiliates to date. The design, permitting, and procurement efforts will take place while the unit is still in operation. The first major construction effort will be to modify the pond operations by relocating the influent lines, minimizing the pond water levels, and isolating flow to a smaller portion of the current 111-acre impoundment operating area will be reduced to approximately 40-50 acres during this effort. This reduction in footprint may require the addition of chemical feeds to provide adequate treatment with the reduction in residence time; however, it will simultaneously allow for continued operation of the plant to maintain generating capacity for the MISO markets and minimize the risk to the environment both by minimizing the potential for any impacts to groundwater and by opening up a significant portion of the remaining impoundment to allow for dewatering, grading, and closure.

Table 6-1 provides estimates for the estimated durations required to close a portion of the pond footprint after the date noted to begin closure construction (Phase 1), as well as the current estimates for the closure of the active area (Phase 2, remaining 40-50 acres). In order to dewater the closure area, Electric Energy will likely release pond water through the existing Outfall 001 and employ pumps as necessary, and potentially an engineered dewatering system such as wellpoints to aid in stabilizing the material. As the water level is lowered and the material is stabilized, the contractor will work across the pond re-grading the existing CCR material to achieve positive drainage. As grading is completed in certain areas, the contractor

may begin placing the final cover system which will consist of an 18-inch infiltration layer and 6-inch erosion layer in accordance with the requirements of the CCR Rule (or an alternative cover system that meets these minimum standards). The schedule for the Phase 1 cover installation will overlap with the Phase 1 grading schedule and is expected to finish approximately two months after the grading effort is completed. Once cover is placed, the area will be seeded and stabilized. The schedule for seeding and stabilizing will overlap with cover installation and finish approximately one month after the cover system is placed. Closure is essentially completed once the erosion control layer is placed, so the final month of this activity will provide additional float to the schedule.

Action	Estimated Timeline (Months)
Spec, bid, and Award Engineering Services for CCR Impoundment Closure	3
Finalize CCR unit closure plan and seek IEPA approval for CCR unit closure	12
 Obtain environmental permits (based on IEPA approval of closure plan): State Waste Pollution Control Construction/Operating Permit NPDES Industrial Wastewater Permit Modification General NPDES Permit for Storm Water Discharges from Construction Site Activities and Storm Water Pollution Prevention Plan (SWPPP) Proposed 35 III. Admin Code 845 operating permit application is due NLT September 2021. Construction permit application is anticipated to 	21
be due NLT July 2022. Spec, bid, and Award Construction Services for CCR	3
Impoundment Closure	5
Begin Construction of Closure Date	October 17, 2025
Minimize Active Area of Impoundment / Dewater Phase 1 Area	6
Regrade CCR Material in Phase 1 Area	12
Install Cover System – Phase 1 area*	7
Establish Vegetation – Phase 1 Area**	2

Table 6-1: Joppa East Ash Pond Closure Schedule

Action	Estimated Timeline (Months)
Cease Placement of Waste	July 17, 2027
Dewater Impoundment – Phase 2 Area	3
Regrade CCR Material – Phase 2 Area	6
Install Cover System – Phase 2 Area	5
Establish Vegetation, Perform Site Restoration Activities, Complete Closure, and Initiate Post-Closure Care**	2
Total Estimated Time to Complete Closure	75 months
Date by Which Closure Must be Complete	October 17, 2028

* Activity expected to overlap with grading operations, finishing 2 months after grading is completed.

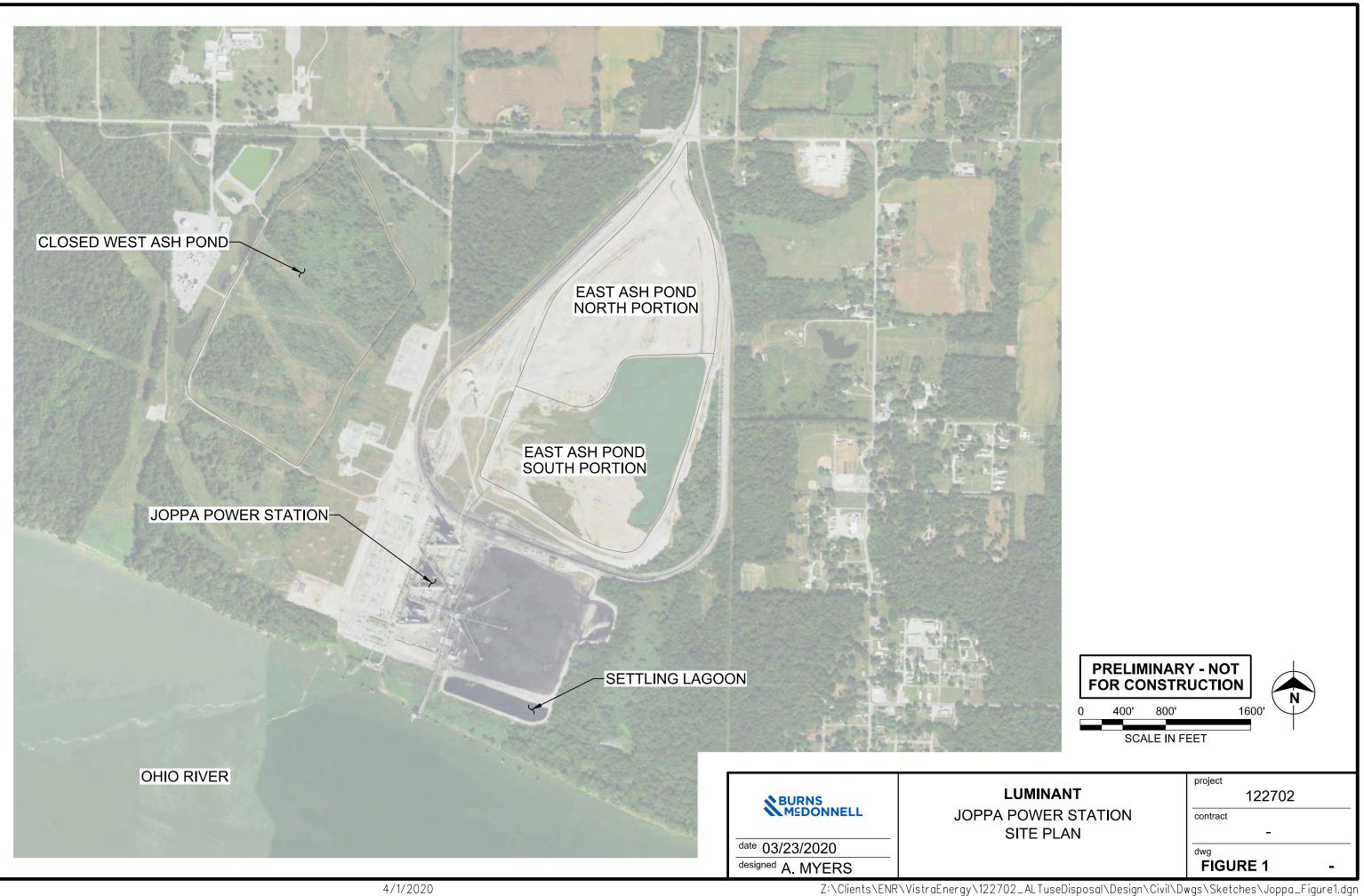
** Activity expected to overlap with cover system installation, finishing 1 month after cover installation is completed.

7.0 CONCLUSION

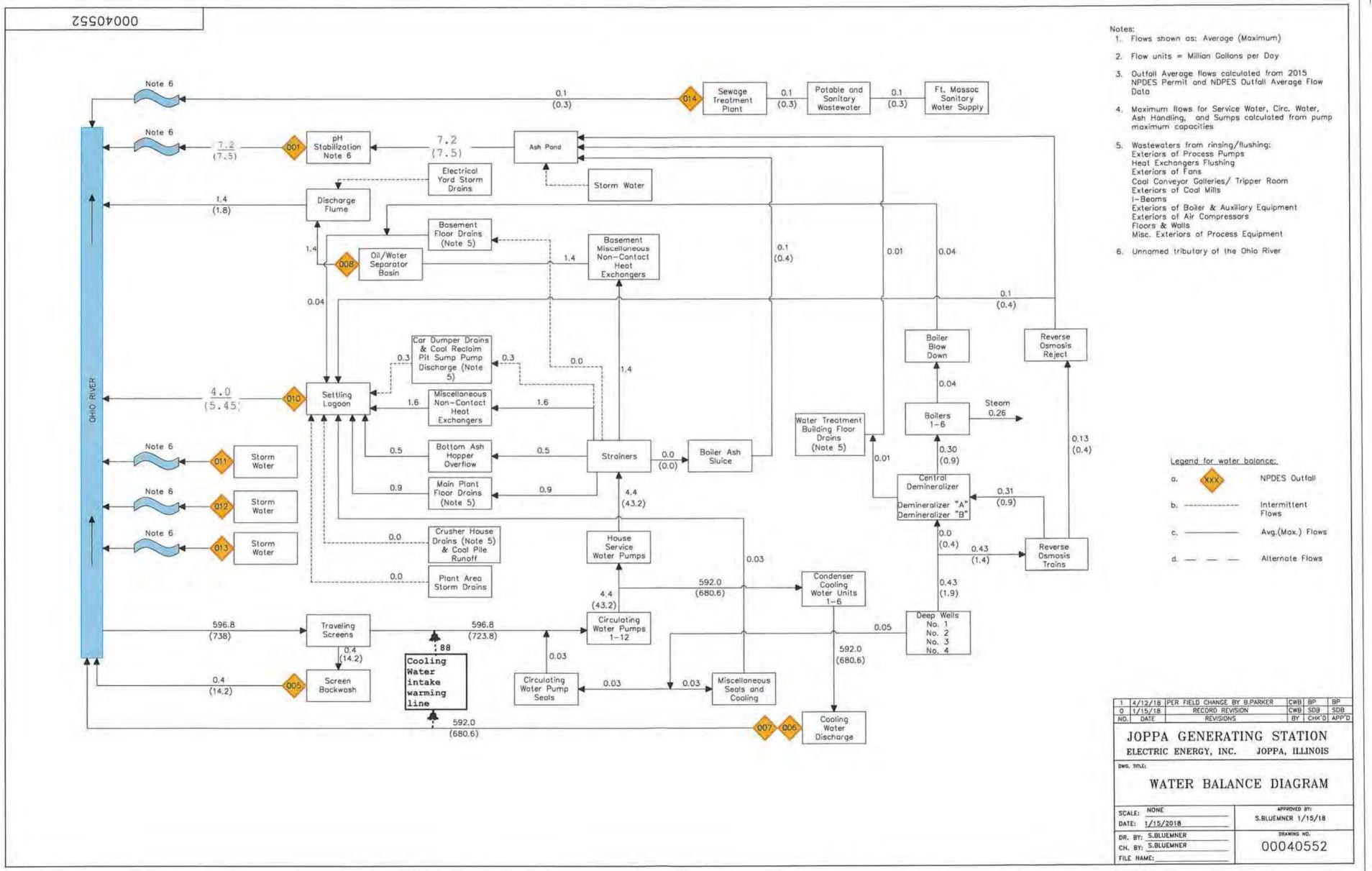
Based upon the information included in and attached to this demonstration, Electric Energy has demonstrated that the requirements of 40 C.F.R. § 257.103(f)(2) are satisfied for the 111-acre East Ash Pond at Joppa. This CCR surface impoundment is needed to continue to manage the CCR and non-CCR wastestreams identified in Section 3.2 and 3.3 above, is larger than 40 acres, and the boilers at the station will cease coal-fired operation and the East Ash Pond will be closed by the October 17, 2028 deadline. Therefore, this CCR unit qualifies for the site-specific alternative deadline for the initiation of closure authorized by 40 C.F.R. § 257.103(f)(2).

Therefore, it is requested that EPA approve Electric Energy's demonstration and authorize the East Ash Pond at Joppa to continue to receive CCR and non-CCR wastestreams notwithstanding the deadline in § 257.101(a)(1) and to grant the alternative deadline of October 17, 2028, by which to complete closure of the impoundment.

APPENDIX A – SITE PLAN



APPENDIX B – WATER BALANCE DIAGRAM



ATTACHMENT 1 – RISK MITIGATION PLAN

RISK MITIGATION PLAN - 40 C.F.R. § 257.103(f)(2)(v)(B)

INTRODUCTION

To demonstrate that the criteria in §40 C.F.R. 257.103(f)(2)(ii) has been met, Electric Energy, Inc. ("Electric Energy") has prepared this Risk Mitigation Plan for the East Ash Pond located at the Joppa Power Station ("Joppa") in Joppa, Illinois.

• EPA is requiring a risk mitigation plan to "address the potential risk of continued operation of the CCR surface impoundment while the facility moves towards closure of their coal-fired boiler(s), to be consistent with the court's holding in *USWAG* that RCRA requires EPA to set minimum criteria for sanitary landfills that prevent harm to either human health or the environment." 85 Fed. Reg. at 53,516, 53,548 (Aug. 28, 2020).

As required by § 257.103(f)(2)(v)(B), the Risk Mitigation Plan must describe the "measures that will be taken to expedite any required corrective action," and contain the three following elements:

- First, "a discussion of any physical or chemical measures a facility can take to limit any future releases to groundwater during operation." § 257.103(f)(2)(v)(B)(1). In promulgating this requirement, EPA explained that this "might include stabilization of waste prior to disposition in the impoundment or adjusting the pH of the impoundment waters to minimize solubility of contaminants [and that] [t]his discussion should take into account the potential impacts of these measures on Appendix IV constituents." 85 Fed. Reg. at 53,548.
- Second, "a discussion of the surface impoundment's groundwater monitoring data and any found exceedances; the delineation of the plume (if necessary based on the groundwater monitoring data); identification of any nearby receptors that might be exposed to current or future groundwater contamination; and how such exposures could be promptly mitigated." § 257.103(f)(2)(v)(B)(2).
- Third, "a plan to expedite and maintain the containment of any contaminant plume that is either present or identified during continued operation of the unit." § 257.103(f)(2)(v)(B)(3). In promulgating this final requirement, EPA explained that "the purpose of this plan is to demonstrate that a plume can be fully contained and to define how this could be accomplished in the most accelerated timeframe feasible to prevent further spread and eliminate any potential for exposures." 85 Fed. Reg. at 53,549. In addition, EPA stated that "this plan will be based on relevant site data, which may include groundwater chemistry, the variability of local hydrogeology, groundwater elevation and flow rates, and the presence of any surface water features that would influence rate and direction of contamination movement. For example, based on the rate and direction of groundwater flow and potential for diffusion of the plume, this plan could identify the design and spacing of extraction wells necessary to prevent further downgradient migration of contaminated groundwater." 85 Fed. Reg. at 53,549.

Consistent with these requirements and guidance, Electric Energy plans to continue to mitigate the risks to human health and the environment from the Joppa East Ash Pond as detailed in this Risk Mitigation Plan.

1 OPERATIONAL MEASURES TO LIMIT FUTURE RELEASES TO GROUNDWATER- 40 C.F.R. § 257.101(F)(2)(V)(B)(1)

The Joppa Pond is a 111-acre CCR surface impoundment. Consistent with the requirements of the CCR rule, compliance documents on Joppa's CCR public website reflect the characterization of the Ash Pond as a single unit for purposes of groundwater monitoring and closure activities.

The Joppa CCR surface impoundment receives CCR transport waters from bottom ash and economizer ash plus non-CCR process waters onsite before discharging to the Ohio River via Outfall 001 in accordance with NPDES Permit No. IL0004171.

At Joppa, none of the Appendix IV parameter have reported SSLs, or SSLs above their respective Ground Water Protection Standards (GWPSs) as sampled and analyzed per the CCR surface impoundment's groundwater monitoring program. Therefore, Joppa's current physical treatment operation adequately limits potential risks to human health and the environment during operation. Joppa will continue this treatment process for the CCR surface impoundment until such time as closure is required per 40 CFR 257. The facility's current physical treatment process is discussed below.

1.1 CURRENT OPERATION OF PHYSICAL TREATMENT

Fly ash and air heater ash are captured dry. Therefore, current operations do not add fly ash transport waters to the CCR surface impoundment.

As part of normal operations, bottom ash and economizer ash are transported through the sluice lines into the CCR surface impoundment where it is dewatered and transported offsite for beneficial reuse. The CCR surface impoundment is also a wastewater treatment settling system which allows the solids to settle.

Therefore, since fly ash transport water is not conveyed to the CCR surface impoundment and bottom ash solids are removed from the CCR surface impoundment, the current operation of Joppa's CCR surface impoundment limits future releases to groundwater during operation, and consequently no potential safety impacts or exposure to human health or environmental receptors are expected to result.

If Appendix IV releases are discovered per the facility's groundwater monitoring program, Electric Energy will test, evaluate, and implement a chemical treatment method (i.e. pH adjustment, coagulation, precipitation, or other method as determined) for the Joppa CCR Impoundment to limit potential risks to human health and the environment during operation

2 GROUNDWATER IMPACTS, RECEPTORS, AND POTENTIAL EXPOSURE MITIGATION - 40 C.F.R. § 257.101(F)(2)(V)(B)(2)

The Joppa East Ash Pond, with a footprint of approximately 111 acres (Figure 1), currently remains in assessment monitoring. There have been no statistically significant levels (SSLs) of Appendix IV parameter concentrations since assessment monitoring was established on May 9, 2018 in accordance with 40 CFR § 257.95. The most recent summary of groundwater monitoring activities is provided in the "2019 Annual Groundwater Monitoring and Corrective Action Report, Joppa East Ash Pond, Joppa Power Station" (Ramboll, 2020) [see Attachment 1]. A summary of the assessment monitoring program is provided in Table 1. Since there have been no SSLs or GWPS exceedances to date, no plume delineation maps have been necessary.

Receptors

Should a release to groundwater for one or more Appendix IV parameters occur in the future, the two primary risks to human health and the environment are via groundwater exposure and surface water exposure. Groundwater exposure would be via ingestion or dermal contact, both of which are likely an incomplete exposure pathway for CCR-related constituents originating from the Joppa East Ash Pond. Impacted groundwater potentially migrating to nearby water wells or surface water bodies – specifically the Ohio River bordering Joppa to the south – could be an exposure pathway but does not pose a risk for the reasons discussed below.

There are no surface-water intakes for community water supply (CWS) on the Ohio River identified within a onemile radius of the Joppa property line. In addition, there are no non-CWS surface water intakes on the Ohio River within 2,500 feet of the site boundary.

There are no potable industrial, commercial, CWS or non-CWS water wells in a downgradient or cross-gradient groundwater flow direction relative to the Joppa East Ash Pond that are at risk of impacts from a release. There is one domestic (private) well within the Village of Joppa that is located potentially downgradient (south) of the East Ash Pond. Although the property on which this well is located may be served by the Village of Joppa's CWS, the possibility that this well may be used for drinking water cannot be ruled out based on available information. However, since there are currently no exceedances of GWPS(s) for Appendix IV parameters in any monitoring wells at the East Ash Pond, the identified well, or any other wells within a 2,500-foot search radius of the Joppa Power Plant, are not currently at risk.

Ambient groundwater flow in the Uppermost Aquifer beneath the East Ash Pond is southward towards the Ohio River. Groundwater elevations vary seasonally and may fluctuate by about 10 feet. Slight seasonal variation in groundwater flow directions ranging from southeast to southwest are also observed; however, the major component of groundwater flow direction is consistently south toward the Ohio River, which is the primary discharge area for groundwater near Joppa (refer to the description of hydrogeology attached to the alternative closure demonstration letter).

Horizontal hydraulic gradients in the Uppermost Aquifer beneath the East Ash Pond typically range from 0.002 to 0.003 ft/ft. Groundwater flow velocity in the Uppermost Aquifer beneath the East Ash Pond ranges from 0.003 to 0.01 feet per day (ft/day) southward towards the Ohio River (refer to the description of hydrogeology attached to the alternative closure demonstration letter).

Exposure Mitigation

Mitigation of future potential exposures to groundwater contamination from continued operation of the Joppa East Ash Pond is discussed in detail in the following section.

3 CONTAMINANT PLUME CONTAINMENT: OPTIONS EVALUATION AND PLAN- 40 C.F.R. § 257.101(F)(2)(V)(B)(3)

Appropriate corrective measure(s) to address future potential impacted groundwater associated with the Joppa East Ash Pond are based on impacts to the Uppermost Aquifer. The Uppermost Aquifer consists of unlithified silty sand, sand, and gravel deposits within the McNairy Formation, which is approximately 85 feet thick near the East Ash Pond. The geometric mean horizontal hydraulic conductivity of the McNairy Formation based on field testing of monitoring wells around the East Ash Pond is 2.4×10^{-4} centimeters per second (cm/s). The overlying material (inclusive of both the Equality and Metropolis Formations) is a confining unit of clay, silty clay, sandy clay, and silt with a measured geometric mean hydraulic conductivity of 5.9×10^{-6} centimeters per second (cm/s). No known wells in the area utilize the Equality and Metropolis Formations for groundwater and most wells obtain groundwater from sands and gravels of the McNairy Formation (i.e. Uppermost Aquifer) or underlying Mississippian-age limestone bedrock, which generally occurs at depths greater than 100 feet (refer to the description of hydrogeology attached to the alternative closure demonstration letter).

Since there has been no release of Appendix IV parameters to groundwater above GWPS(s), which would trigger a Corrective Measures Assessment (CMA) under 40 C.F.R. § 257.96 based on specific parameter concentrations and contaminant plume dimensions, several options are evaluated to address potential future plume containments. The evaluation criteria for assessing remedial options are the following: performance; reliability; ease of implementation; potential impacts of the remedies (safety, cross-media, and control of exposure to residual contamination); time required to begin and complete the remedy; and, institutional requirements that may substantially affect implementation of the remedy(s), such as permitting, environmental or public health requirements.

Although future potential source control measures (e.g. closure in place, closure by removal to on-site or off-site landfill, in-situ solidification/stabilization) to mitigate groundwater impacts are typically considered as part of a CMA process upon closure of the Joppa East Ash Pond, the shorter-term options considered for mitigating groundwater impacts relative to a potential future release of one or more Appendix IV parameters at Joppa are as follows:

- Monitored Natural Attenuation (MNA)
- Groundwater Cutoff Wall
- In-Situ Chemical Treatment
- Permeable Reactive Barrier
- Groundwater Extraction

These same groundwater remedial corrective measures will be evaluated for all Appendix IV constituents that present a future risk to human health or the environment.

Monitored Natural Attenuation (MNA)

Upon notification of a release of one or more Appendix IV constituent(s) to groundwater, MNA will be evaluated with site-specific characterization data and geochemical analysis as a long term remedial option, combined with source control measures, through application of the USEPA's tiered approach to MNA (USEPA 1999, 2007 and 2015):

1. Demonstrate that the area of groundwater impacts is not expanding.

- 2. Determine the mechanisms and rates of attenuation.
- 3. Determine that the capacity of the aquifer is sufficient to attenuate the mass of constituents in groundwater and that the immobilized constituents are stable and will not remobilize.
- 4. Design a performance monitoring program based on the mechanisms of attenuation and establish contingency remedies (tailored to site-specific conditions) should MNA not perform adequately.

MNA is not regarded as a short-term remedial option for contaminant plume containment, but as a potential long-term option following implementation of shorter term control measures.

Groundwater Extraction

This corrective measure includes installation of a series of groundwater pumping wells or trenches to control and extract impacted groundwater. Groundwater extraction captures and contains impacted groundwater and can limit plume expansion and/or off-site migration. Construction of a groundwater extraction system typically includes, but is not limited to, the following primary project components:

- Designing and constructing a groundwater extraction system consisting of a series of extraction wells or trenches located around the perimeter of the contaminant plume and operating at a rate to allow capture of CCR impacted groundwater.
- Designing a system to manage extracted groundwater, which may include modification to the existing NPDES permit, including treatment prior to discharge, if necessary.
- Ongoing inspection and maintenance of the groundwater extraction system.

Installation of a groundwater extraction system, whether wells or trenches, can be expedited with the assumption that there is a good conceptual site model (CSM) of the hydrogeological system around the CCR unit, groundwater flow and transport model, and aquifer test if a well system is the best option for intercepting the groundwater contaminant plume. Upon notification of an SSL exceedance of a GWPS for one or more Appendix IV parameters, an aquifer test will be conducted, and groundwater model developed for designing a groundwater extraction system for optimization of contaminant plume capture.

A schematic of a typical groundwater extraction well is shown on Figure 2. Based on site specific hydrogeology and future potential plume width and depth, a groundwater extraction system will typically consist of one to three extraction wells with pitless adapter's manifolded together with HDPE conveyance pipe to a common tank or lined collection vault prior to treatment at the on-site wastewater treatment plant and discharge via the NPDES permitted outfall.

Groundwater Cutoff Wall

Vertical cutoff walls are used to control and/or isolate impacted groundwater. Low permeability cutoff walls can be used to prevent horizontal off-site migration of potentially impacted groundwater. Cutoff walls act as barriers to migration of impacted groundwater and can isolate soils that have been impacted by CCR to prevent contact with unimpacted groundwater. Cutoff walls are often used in conjunction with an interior pumping system to establish a reverse gradient within the cutoff wall. The reverse gradient maintains an inward flow through the wall, keeping it from acting as a groundwater dam and controlling potential end-around or breakout flow of contaminated groundwater.

A commonly used cutoff wall construction technology is the slurry trench method, which consists of excavating a trench and backfilling it with a soil-bentonite mixture, often created with the soils excavated from the trench. The

trench is temporarily supported with bentonite slurry that is pumped into the trench as it is excavated. Excavation for cutoff walls is conducted with conventional hydraulic excavators, hydraulic excavators equipped with specialized booms to extend their reach (*i.e.*, long-stick excavators), or chisels and clamshells, depending upon the depth of the trench and the material to be excavated. For a cutoff wall to be technically feasible, there must be a low-permeability lower confining layer into which the barrier can be keyed, and it must be at a technically feasible depth.

Permeable Reactive Barrier

Chemical treatment via a Permeable Reactive Barrier (PRB) is defined as an emplacement of reactive materials in the subsurface designed to intercept a contaminant plume, provide a flow path through the reactive media, and transform or otherwise render the contaminant(s) into environmentally acceptable forms to attain remediation concentration goals downgradient of the barrier (EPRI, 2006).

As groundwater passes through the PRB under natural gradients, dissolved constituents in the groundwater react with the media and are transformed or immobilized. A variety of media have been used or proposed for use in PRBs. Zero-valent iron has been shown to effectively immobilize CCR constituents, including arsenic, chromium, cobalt, molybdenum, selenium and sulfate. Zero-valent iron has not been proven effective for boron, antimony, or lithium (EPRI, 2006).

System configurations include continuous PRBs, in which the reactive media extends across the entire path of the contaminant plume; and funnel-and-gate systems, where barrier walls are installed to control groundwater flow through a permeable gate containing the reactive media. Continuous PRBs intersect the entire contaminant plume and do not materially impact the groundwater flow system. Design may or may not include keying the PRB into a low-permeability unit at depth. Funnel-and-gate systems utilize a system of barriers to groundwater flow (funnels) to direct the contaminant plume through the reactive gate. The barriers, typically some form of cutoff wall, are keyed into a low-permeability unit at depth to prevent short circuiting of the plume. Funnel-and-gate design must consider the residence time to allow chemical reactions to occur. Directing the contaminant plume through the reactive gate can significantly increase the flow velocity, thus reducing residence time.

Design of PRB systems requires rigorous site investigation to characterize the site hydrogeology and to delineate the contaminant plume. A thorough understanding of the geochemical and redox characteristics of the plume is critical to assess the feasibility of the process and select appropriate reactive media. Laboratory studies, including batch studies and column studies using samples of site groundwater, are needed to determine the effectiveness of the selected reactive media at the site (EPRI, 2006).

This is a potential viable option for groundwater corrective measures, to be evaluated further, but is not a short-term solution that can be implemented expeditiously.

In-Situ Chemical Treatment

In-situ chemical treatment for inorganics are being tested and applied with increasing frequency. In-situ chemical treatment includes the targeted injection of reactive media into the subsurface to mitigate groundwater impacts. Inorganic contaminants are typically remediated through immobilization by reduction or oxidation followed by precipitation or adsorption (EPRI, 2006). Chemical reactants that have been applied or are in development for application in treating inorganic contaminants include ferrous sulfate, nanoscale zero-valent iron, organo-phosphorus nutrient mixture (PrecipiPHOS[™]) and sodium dithionite (EPRI, 2006). Zero-valent iron has been shown to effectively immobilize cobalt and molybdenum. Implementation of in-situ chemical treatment requires detailed technical analysis of field hydrogeological and geochemical conditions along with laboratory studies.

This is a potential viable option for groundwater corrective measures, to be evaluated further, but is not a short-term solution that can be implemented expeditiously.

3.1 CONTAINMENT PLAN

Based on the options evaluated for containment of a future potential groundwater contaminant plume originating from the Joppa East Ash Pond for one or more Appendix IV constituents exceeding their GWPS(s), the most viable short-term option of those evaluated is a groundwater extraction well or recovery trench system, which would allow for capture of impacted groundwater and prevention of further plume migration towards the principal potential receptors, which have been identified as the Ohio River and one domestic (private) well within the Village of Joppa located potentially downgradient of the East Ash Pond.

In circumstances where there is not an immediate concern of endangerment to human health or the environment, other longer-term corrective measures may be more viable. The principal method under consideration for controlling potential future Appendix IV constituent releases is MNA. MNA is a potentially viable corrective measure that will be further evaluated for use at the Joppa East Ash Pond.

Depending on the location, depth, and plume geometry of any future potential Appendix IV exceedances of GWPSs, the specific constituent(s) with exceedances, and distance from potential receptors, the other groundwater corrective measures discussed as part of the corrective options evaluation – groundwater cutoff wall, permeable reactive barrier, and in-situ chemical treatment – are all secondary remedial alternatives available for consideration following the current primary options of groundwater extraction for short-term application and MNA for long-term application.

4 **References**

Electric Power Research Institute (EPRI), 2006. Groundwater Remediation of Inorganic Constituents at Coal Combustion Product Management Sites, Overview of Technologies, Focusing on Permeable Reactive Barriers. Electric Power Research Institute, Palo Alto, California. Final Report 1012584, October 2006.

Ramboll, 2020. 2019 Annual Groundwater Monitoring and Corrective Action Report, Joppa East Ash Pond, Joppa Power Station. January 31, 2020.

USEPA, 1999. Use of Monitored Natural Attenuation at Superfund, RCRA Corrective Action, and Underground Storage Tank Sites. Directive No. 9200.U-17P. Washington, D.C.: EPA, Office of Solid Waste and Emergency Response.

USEPA, 2007. Monitored Natural Attenuation of Inorganic Contaminants in Ground Water, Volume 1 – Technical Basis for Assessment. EPA/600/R-07/139. National Risk Management Research Laboratory, Office of Research and Development, U.S. Environmental Protection Agency, Cincinnati, Ohio. October 2007.

USEPA, 2015. Use of Monitored Natural Attenuation for Inorganic Contaminants in Groundwater at Superfund Sites. Directive No. 9283.1-36. U.S. Environmental Protection Agency, Office of Solid Waste and Emergency Response. August 2015.

TABLES

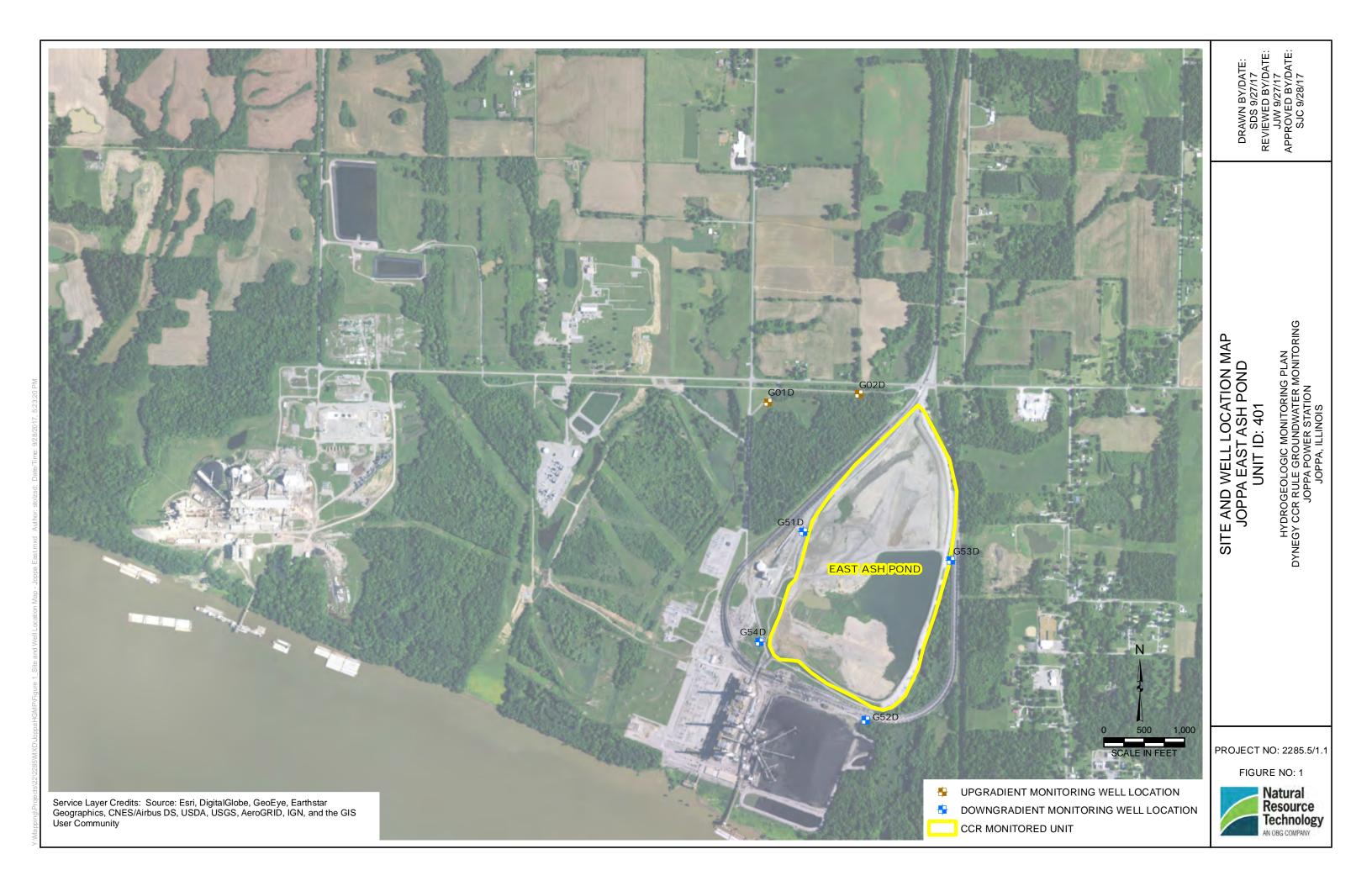
Table 1 - Assessment Monitoring Program Summary, Joppa East Ash Pond

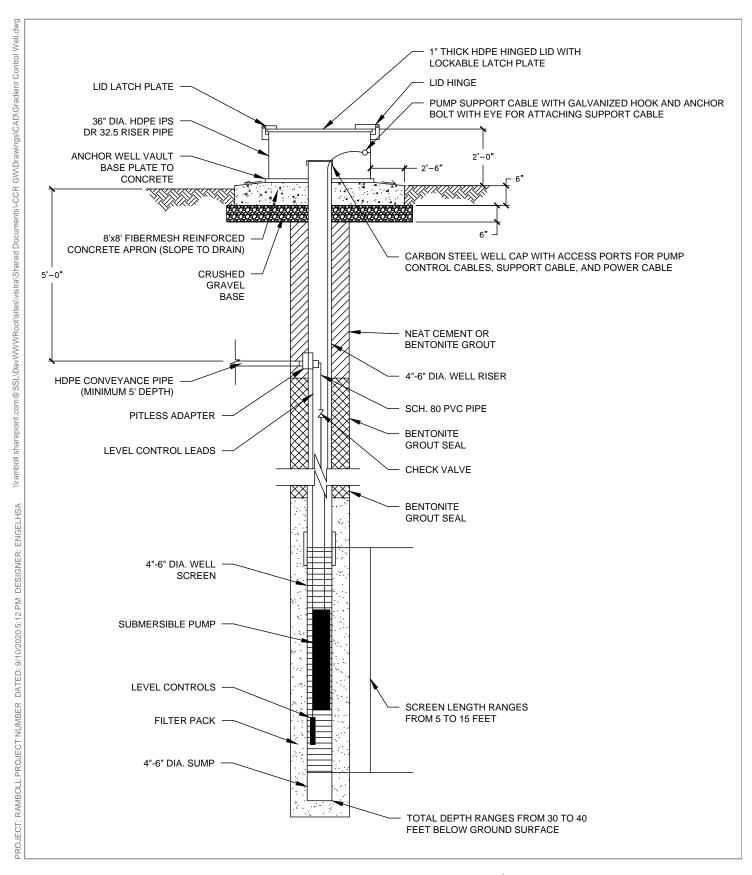
Sampling Dates	Analytical Data Receipt Date	Parameters Collected	SSL(s) Appendix IV	SSL(s) Determination Date	ASD Completion Date	CMA Completion / Status
June 19, 2018	August 3, 2018	Appendix III Appendix IV	NA	NA	NA	NA
September 5, 2018	October 23, 2018	Appendix III Appendix IV Detected ¹	None	January 7, 2019	NA	NA
March 27, 2019	April 30, 2019	Appendix III Appendix IV	None	July 29, 2019	NA	NA
September 9, 2019	October 15, 2019	Appendix III Appendix IV Detected ¹	None	January 13, 2020	NA	NA
March 30, 2020	April 28, 2020	Appendix III Appendix IV Detected	None	July 27, 2020	NA	NA
						[O: RAB 9/11/20; C: EJT 9/15/20

Notes: CMA = Corrective Measures Assessment NA = Not Applicable 1. Groundwater sample analysis was limited to Appendix IV parameters detected in previous events in accordance with 40 C.F.R. Part 257.95(d)(1).



FIGURES





NOTES

1. NOT TO SCALE

TYPICAL HYDRAULIC GRADIENT CONTROL WELL DETAIL

FIGURE 2

RAMBOLL US CORPORATION A RAMBOLL COMPANY

Electric Energy, Inc. JOPPA EAST ASH POND JOPPA, ILLINOIS



ATTACHMENT 1

Prepared for Electric Energy, Inc. Document type 2019 Annual Groundwater Monitoring and Corrective Action Report Date January 31, 2020

2019 ANNUAL GROUNDWATER MONITORING AND CORRECTIVE ACTION REPORT JOPPA EAST ASH POND, JOPPA POWER STATION



2019 ANNUAL GROUNDWATER MONITORING AND CORRECTIVE ACTION REPORT JOPPA EAST ASH POND, JOPPA POWER STATION

Project name	Joppa Power Station
Project no.	72757
Recipient	Electric Energy, Inc.
Document type	Annual Groundwater Monitoring and Corrective Action Report
Version	FINAL
Date	January 31, 2020
Prepared by	Kristen L. Theesfeld
Checked by	Nathaniel R. Keller
Approved by	Eric J. Tlachac
Description	Annual Report in Support of the CCR Rule Groundwater Monitoring Program

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4.	Problems Encountered and Actions to Resolve the Problems	8
5.	Key Activities Planned for 2020	9
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Table A	2018-2019 Assessment Monitoring Program Summary (in text)
Table 1	2019 Analytical Results – Groundwater Elevation and Appendix III Parameters
Table 2	2019 Analytical Results – Appendix IV Parameters
Table 3	Statistical Background Values
Table 4	Groundwater Protection Standards

FIGURES

Figure 1 Monitoring Well Location Map

2019 Annual Groundwater Monitoring and Corrective Action Report Joppa East Ash Pond, Joppa Power Station

ACRONYMS AND ABBREVIATIONS

CCR	Coal Combustion Residuals
EAP	East Ash Pond
GWPS	Groundwater Protection Standard
SAP	Sampling and Analysis Plant
SSL	Statistically Significant Level

EXECUTIVE SUMMARY

This report has been prepared to provide the information required by Title 40 of the Code of Federal Regulations (40 C.F.R.) § 257.90(e) for Joppa East Ash Pond (EAP) located at Joppa Power Station near Joppa, Illinois.

Groundwater is being monitored at Joppa EAP in accordance with the Assessment Monitoring Program requirements specified in 40 C.F.R. § 257.95.

No changes were made to the monitoring system in 2019 (no wells were installed or decommissioned).

No Statistically Significant Levels (SSLs) of 40 C.F.R. Part 257 Appendix IV parameters were determined in 2019 and Joppa EAP remains in the Assessment Monitoring Program.

1. INTRODUCTION

This report has been prepared by Ramboll on behalf of Electric Energy, Inc., to provide the information required by 40 C.F.R.§ 257.90(e) for Joppa EAP located at Joppa Power Station near Joppa, Illinois.

In accordance with 40 C.F.R. § 257.90(e), the owner or operator of a Coal Combustion Residuals (CCR) unit must prepare an Annual Groundwater Monitoring and Corrective Action Report for the preceding calendar year that documents the status of the Groundwater Monitoring and Corrective Action Program for the CCR unit, summarizes key actions completed, describes any problems encountered, discusses actions to resolve the problems, and projects key activities for the upcoming year. At a minimum, the Annual Report must contain the following information, to the extent available:

- 1. A map, aerial image, or diagram showing the CCR unit and all background (or upgradient) and downgradient monitoring wells, to include the well identification numbers, that are part of the groundwater monitoring program for the CCR unit.
- 2. Identification of any monitoring wells that were installed or decommissioned during the preceding year, along with a narrative description of why those actions were taken.
- 3. In addition to all the monitoring data obtained under §§ 257.90 through 257.98, a summary including the number of groundwater samples that were collected for analysis for each background and downgradient well, the dates the samples were collected, and whether the sample was required by the Detection Monitoring or Assessment Monitoring Programs.
- 4. A narrative discussion of any transition between monitoring programs (e.g., the date and circumstances for transitioning from Detection Monitoring to Assessment Monitoring in addition to identifying the constituent(s) detected at a Statistically Significant Increase relative to background levels).
- 5. Other information required to be included in the Annual Report as specified in §§ 257.90 through 257.98.

This report provides the required information for Joppa EAP for calendar year 2019.

2. MONITORING AND CORRECTIVE ACTION PROGRAM STATUS

No changes have occurred to the Monitoring Program status in calendar year 2019, and Joppa EAP remains in the Assessment Monitoring Program in accordance with 40 C.F.R. § 257.95.

3. KEY ACTIONS COMPLETED IN 2019

The Assessment Monitoring Program is summarized in Table A. The groundwater monitoring system, including the CCR unit and all background and downgradient monitoring wells is presented in Figure 1. No changes were made to the monitoring system in 2019 (no wells were installed or decommissioned). In general, one groundwater sample was collected from each background and downgradient well during each monitoring event. All samples were collected and analyzed in accordance with the Sampling and Analysis Plan (SAP) (NRT/OBG, 2017a). All monitoring data obtained under 40 C.F.R. §§ 257.90 through 257.98 (as applicable) in 2019 are presented in Tables 1 and 2. Analytical data were evaluated in accordance with the Statistical Analysis Plan (NRT/OBG, 2017b) to determine any SSLs of Appendix IV parameters over Groundwater Protection Standards (GWPSs).

Statistical background values are provided in Table 3 and GWPSs in Table 4.

Analytical results for the June and September 2018 sampling events were provided in the 2018 Annual Groundwater Monitoring and Corrective Action Report.

Sampling Dates	Analytical Data Receipt Date	Parameters Collected	SSL(s)	SSL(s) Determination Date
June 19, 2018	October 10, 2018	Appendix III Appendix IV	NA	NA
September 5, 2018	October 10, 2018	Appendix III Appendix IV Detected 1	None	January 7, 2019
March 27, 2019	April 15, 2019	Appendix III Appendix IV	None	July 17, 2019
September 9, 2019	October 15, 2019	Appendix III Appendix IV Detected ¹	NA	TBD

Table A – 2018-2019 Assessment Monitoring Program Summary

Notes:

NA: Not Applicable

TBD: To Be Determined

1. Groundwater sample analysis was limited to Appendix IV parameters detected in previous events in accordance with 40 C.F.R. § 257.95(d)(1).

4. PROBLEMS ENCOUNTERED AND ACTIONS TO RESOLVE THE PROBLEMS

No problems were encountered with the Groundwater Monitoring Program during 2019. Groundwater samples were collected and analyzed in accordance with the SAP (NRT/OBG, 2017a), and all data were accepted.

5. KEY ACTIVITIES PLANNED FOR 2020

The following key activities are planned for 2020:

- Continuation of the Assessment Monitoring Program with semi-annual sampling scheduled for the first and third quarters of 2020.
- Complete evaluation of analytical data from the downgradient wells, using GWPSs to determine whether an SSL of Appendix IV parameters has occurred.
- If an SSL is identified, potential alternate sources (i.e., a source other than the CCR unit caused the SSL or that that SSL resulted from error in sampling, analysis, statistical evaluation, or natural variation in groundwater quality) will be evaluated.
 - If an alternate source is demonstrated to be the cause of the SSL, a written demonstration will be completed within 90 days of SSL determination and included in the 2020 Annual Groundwater Monitoring and Corrective Action Report.
 - If an alternate source(s) is not identified to be the cause of the SSL, the applicable requirements of 40 C.F.R. §§ 257.94 through 257.98 (e.g., assessment of corrective measures) as may apply in 2020 will be met, including associated recordkeeping/notifications required by 40 C.F.R. §§ 257.105 through 257.108.

6. **REFERENCES**

Natural Resource Technology, an OBG Company (NRT/OBG), 2017a. Sampling and Analysis Plan, Joppa East Ash Pond, Joppa Power Station, Joppa, Illinois, Project No. 2285, Revision 0, October 17, 2017.

Natural Resource Technology, an OBG Company (NRT/OBG), 2017b, Statistical Analysis Plan, Joppa Power Station, Electric Energy, Inc., October 17, 2017.

TABLES

TABLE 1.

2019 ANALYTICAL RESULTS - GROUNDWATER ELEVATION AND APPENDIX III PARAMETERS 2019 ANNUAL GROUNDWATER MONITORING AND CORRECTIVE ACTION REPORT JOPPA POWER STATION

UNIT ID 401 - JOPPA EAST ASH POND

JOPPA, ILLINOIS

ASSESSMENT MONITORING PROGRAM

								40 C.F.R.	Part 257 App	endix III		
Well Identification Number	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)	Date & Time Sampled	Depth to Groundwater (ft) ¹	Groundwater Elevation (ft NAVD88)	Boron, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Fluoride, total (mg/L)	pH (field) (S.U.)	Sulfate, total (mg/L)	Total Dissolved Solids (mg/L)
						6020A ²	6020A ²	9251 ²	9214 ²	SM 4500 H+B ²	9036 ²	SM 2540C ²
Background /	Upgradient Mo	nitoring Wells										
G01D	37.220429	-88.857179	3/27/2019 17:20	34.50	329.69	<0.025	25.1	8	0.23	6.7	30	310
GOID	37.220429	-88.85/1/9	9/9/2019 17:37	42.75	321.44	<0.025	25.6	8	0.23	6.4	37	336
G02D	37.220715	-88.853311	3/27/2019 16:33	34.43	329.22	0.0473	38.7	20	0.20	6.6	20	262
GO2D	37.220715	-88.853311	9/9/2019 17:08	42.37	321.28	0.0429	40.3	18	0.21	6.5	20	264
Downgradient	t Monitoring We	ells										
G51D	37.216016	-88.855653	3/27/2019 16:07	34.57	329.28	0.778	34.7	6	<0.10	5.7	125	350
6510	37.210010	-00.000000	9/9/2019 16:43	43.83	320.02	0.501	31.3	6	<0.10	5.3	109	320
G52D	37.209626	-88.852943	3/27/2019 17:49	19.68	328.73	<0.025	59.8	13	0.28	6.4	81	376
G32D	37.209020	-00.032943	9/9/2019 18:08	22.92	325.49	<0.025	52.2	14	0.27	6.0	78	370
G53D	37.215069	-88.849367	3/27/2019 15:00	27.20	328.27	0.269	30.5	12	0.59	6.6	54	272
G33D	37.213009	-00.049307	9/9/2019 15:12	36.99	318.48	0.385	42.2	18	0.67	6.2	80	364
G54D	37.212264	-88.857485	3/27/2019 15:45	30.73	326.30	1.03	115	22	0.35	6.8	142	510
654D	37.212204	-00.03/485	9/9/2019 16:00	42.95	314.08	0.614	79.9	<25	0.32	6.4	136	482

Notes:

40 C.F.R. = Title 40 of the Code of Federal Regulations

ft = foot/feet

mg/L = milligrams per liter

NAVD88 = North American Vertical Datum of 1988

S.U. = Standard Units

< = concentration is less than the concentration shown, which corresponds to the reporting limit for the method; estimated concentrations below the reporting limit and associated qualifiers are not provided since not

utilized in statistics to determine Statistically Significant Increases (SSIs) over background.

¹All depths to groundwater were measured on the first day of the sampling event.

²4-digit numbers represent SW-846 analytical methods.



TABLE 2. 2019 ANALYTICAL RESULTS - APPENDIX IV PARAMETERS

2019 ANNUAL GROUNDWATER MONITORING AND CORRECTIVE ACTION REPORT

JOPPA POWER STATION

UNIT ID 401 - JOPPA EAST ASH POND

JOPPA, ILLINOIS

ASSESSMENT MONITORING PROGRAM

										40 C.F.I	R. Part 257 Ap	pendix IV							
Well Identification Number	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)	Date & Time Sampled	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Cadmium, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	Radium 226/228, Combined (pCi/L)	Selenium, total (mg/L)	Thallium, total (mg/L)	
				6020A ¹	6020A ¹	6020A ¹	6020A ¹	6020A ¹	6020A ¹	6020A ¹	6020A ¹	6020A ¹	6020A ¹	7470A ¹	6020A ¹	903/904 ¹	6020A ¹	6020A ¹	
Background /	' Upgradient M	onitoring Wells	6																
G01D	37.220429	-88.857179	3/27/2019 17:20	<0.0010	<0.0010	0.129	<0.0010	<0.0010	0.0030	0.0014	0.23	<0.0010	0.0015	<0.00020	<0.0015	0.78	0.0015	<0.0020	
	37.220429	-00.037179	9/9/2019 17:37 ²	NA	<0.0010	0.123	NA	NA	0.0044	0.0014	0.23	0.0012	<0.0030	NA	NA	0.79	0.0011	NA	
G02D	37.220715	-88.853311	3/27/2019 16:33	<0.0010	<0.0010	0.235	<0.0010	<0.0010	0.0026	<0.0010	0.20	<0.0010	<0.0015	<0.00020	< 0.0015	0.12	0.0030	<0.0020	
GO2D	37.220715	-00.000011	9/9/2019 17:08 ²	NA	<0.0010	0.208	NA	NA	<0.0015	<0.0010	0.21	<0.0010	<0.0030	NA	NA	0.49	0.0021	NA	
Downgradien	t Monitoring W	/ells																	
G51D	37.216016	-88.855653	3/27/2019 16:07	<0.0010	<0.0010	0.0495	<0.0010	<0.0010	0.0016	0.0026	<0.10	<0.0010	0.0059	<0.00020	<0.0015	0.23	0.0050	<0.0020	
GSTD	37.210010	-88.855055	9/9/2019 16:43 ²	NA	<0.0010	0.0377	NA	NA	<0.0015	0.0017	<0.10	<0.0010	0.0057	NA	NA	0.36	0.0042	NA	
G52D	37.209626	-88.852943	3/27/2019 17:49	<0.0010	0.0064	0.271	<0.0010	<0.0010	<0.0015	0.0069	0.28	<0.0010	0.0028	<0.00020	< 0.0015	0.58	<0.0010	<0.0020	
G32D	37.209020	-00.032943	9/9/2019 18:08 ²	NA	0.0021	0.254	NA	NA	<0.0015	0.0022	0.27	<0.0010	<0.0030	NA	NA	1.54	<0.0010	NA	
G53D	37.215069	00 040247	-88.849367	3/27/2019 15:00	<0.0010	<0.0010	0.101	<0.0010	<0.0010	<0.0015	<0.0010	0.59	<0.0010	<0.0015	<0.00020	<0.0015	0.17	<0.0010	<0.0020
G33D	37.215009	-00.049307	9/9/2019 15:12 ²	NA	<0.0010	0.128	NA	NA	<0.0015	0.0020	0.67	<0.0010	<0.0030	NA	NA	0.03	<0.0010	NA	
G54D	37.212264	-88.857485	3/27/2019 15:45	<0.0010	0.0011	0.120	<0.0010	<0.0010	<0.0015	0.0138	0.35	<0.0010	0.0037	<0.00020	<0.0015	0.42	<0.0010	<0.0020	
034D	57.212204	-00.037403	9/9/2019 16:00 ²	NA	<0.0010	0.128	NA	NA	<0.0015	0.0117	0.32	<0.0010	0.0037	NA	NA	0.84	<0.0010	NA	

[O: RAB 12/23/19, C: KLT 12/24/19]

Notes:

40 C.F.R. = Title 40 of the Code of Federal Regulations

mg/L = milligrams per liter

NA = Not Analyzed

pCi/L = picoCuries per liter

< = concentration is less than concentration shown, which corresponds to the reporting limit for the method; estimated concentrations below the reporting limit and associated qualifiers are not provided since not utilized in statistics to determine

Statistically Significant Levels (SSLs) over Groundwater Protection Standards.

¹4-digit numbers represent SW-846 analytical methods and 3-digit numbers represent Clean Water Act analytical methods.

²Only the parameters detected during the previous sampling events were analyzed during this sampling event, in accordance with 40 C.F.R. § 257.95(d)(1).

TABLE 3. STATISTICAL BACKGROUND VALUES 2019 ANNUAL GROUNDWATER MONITORING AND CORRECTIVE ACTION REPORT JOPPA POWER STATION UNIT ID 401 - JOPPA EAST ASH POND JOPPA, ILLINOIS ASSESSMENT MONITORING PROGRAM

Parameter	Statistical Background Value (UPL)
40 C.F.R. Part 257 A	ppendix III
Boron (mg/L)	0.06
Calcium (mg/L)	46
Chloride (mg/L)	29
Fluoride (mg/L)	0.28
pH (S.U.)	6.2 / 6.9
Sulfate (mg/L)	180
Total Dissolved Solids (mg/L)	526
[O: RAB 1	2/23/19, C: KLT 12/24/19]

Notes:

40 C.F.R. = Title 40 of the Code of Federal Regulations

mg/L = milligrams per liter

S.U. = Standard Units

UPL = Upper Prediction Limit





TABLE 4.GROUNDWATER PROTECTION STANDARDS2019 ANNUAL GROUNDWATER MONITORING AND CORRECTIVE ACTION REPORTJOPPA POWER STATIONUNIT ID 401 - JOPPA EAST ASH PONDJOPPA, ILLINOISASSESSMENT MONITORING PROGRAM

Parameter	Groundwater Protection Standard ¹				
40 C.F.R. Part 25	7 Appendix IV				
Antimony (mg/L)	0.006				
Arsenic (mg/L)	0.010				
Barium (mg/L)	2				
Beryllium (mg/L)	0.004				
Cadmium (mg/L)	0.005				
Chromium (mg/L)	0.10				
Cobalt (mg/L)	0.037				
Fluoride (mg/L)	4				
Lead (mg/L)	0.015				
Lithium (mg/L)	0.040				
Mercury (mg/L)	0.002				
Molybdenum (mg/L)	0.10				
Radium 226+228 (pCi/L)	5				
Selenium (mg/L)	0.05				
Thallium (mg/L)	0.002				

[O: RAB 12/23/19, C: KLT 12/24/19]

Notes:

40 C.F.R. = Title 40 of the Code of Federal Regulations

mg/L = milligrams per liter

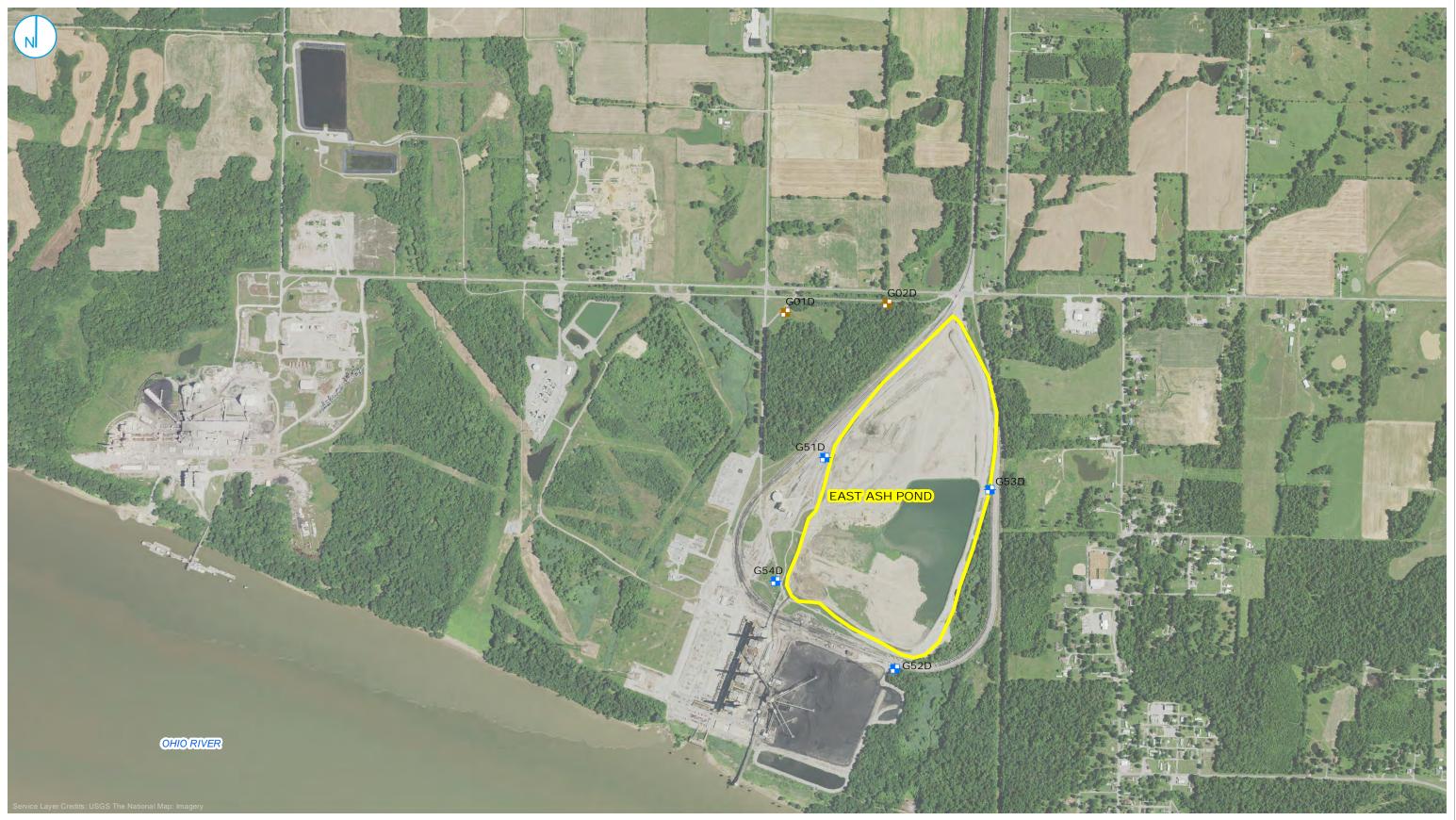
pCi/L = picoCuries per liter

¹Groundwater Protection Standard is the higher of the Maximum Contaminant Level /

Health-Based Level or background.



FIGURES



UPGRADIENT MONITORING WELL LOCATION

- DOWNGRADIENT MONITORING WELL LOCATION
- CCR MONITORED UNIT

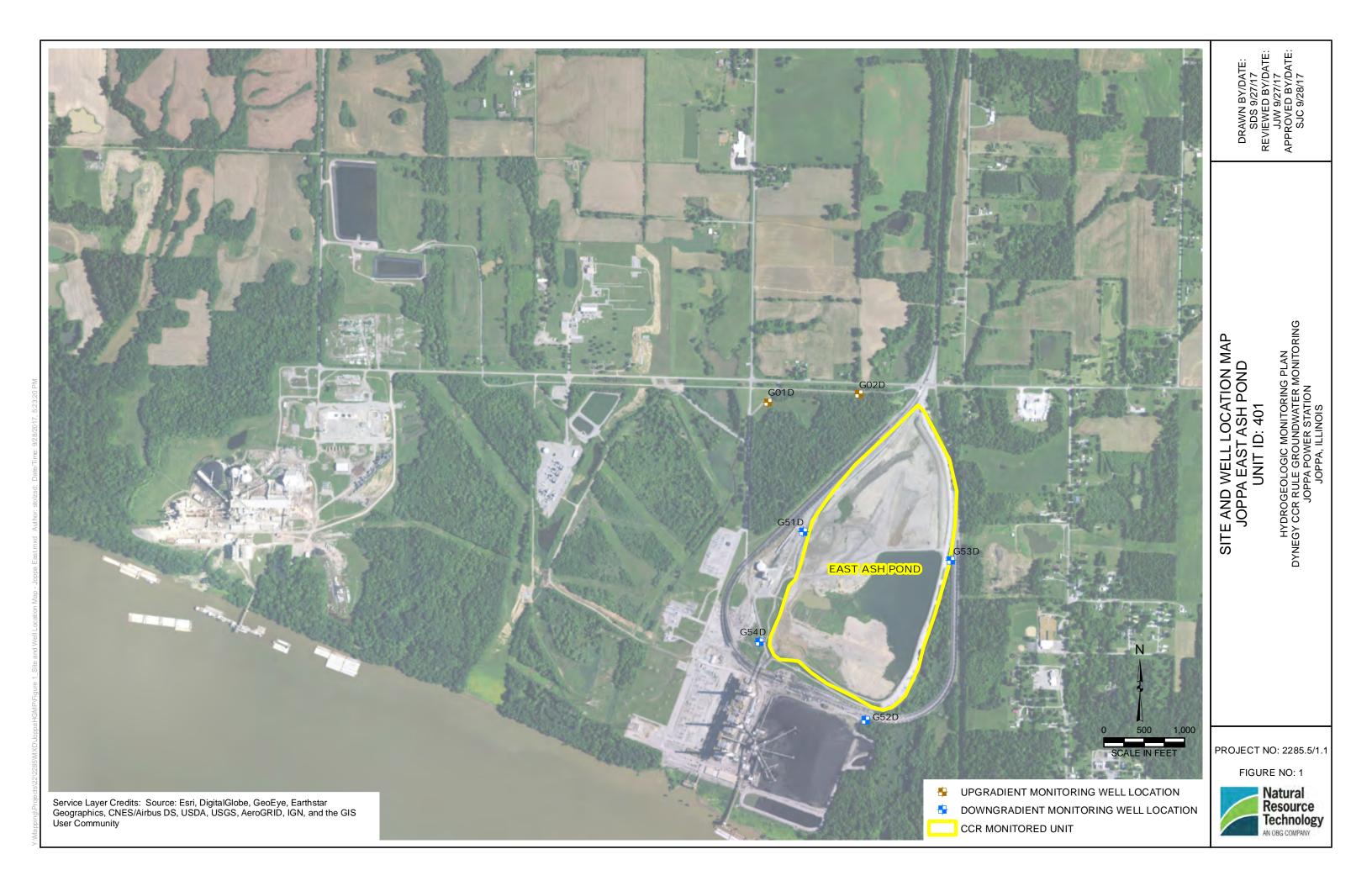
FIGURE 1

O'BRIEN & GERE ENGINEERS, INC. A RAMBOLL COMPANY



MONITORING WELL LOCATION MAP JOPPA EAST ASH POND **UNIT ID:401**

ATTACHMENT 2 – MAP OF GROUNDWATER MONITORING WELL LOCATIONS

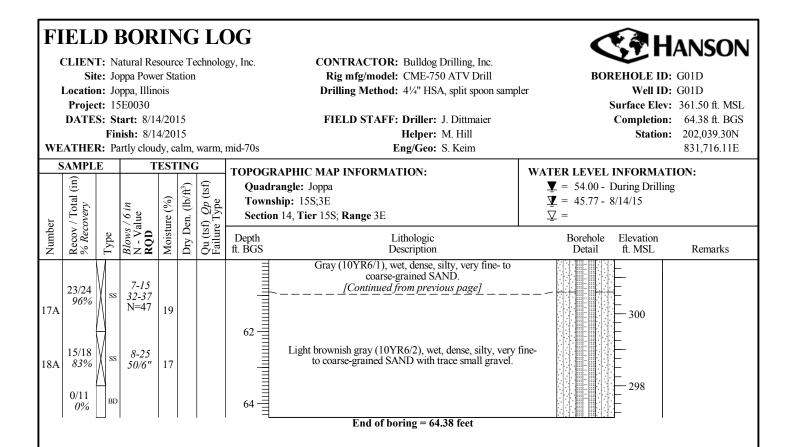


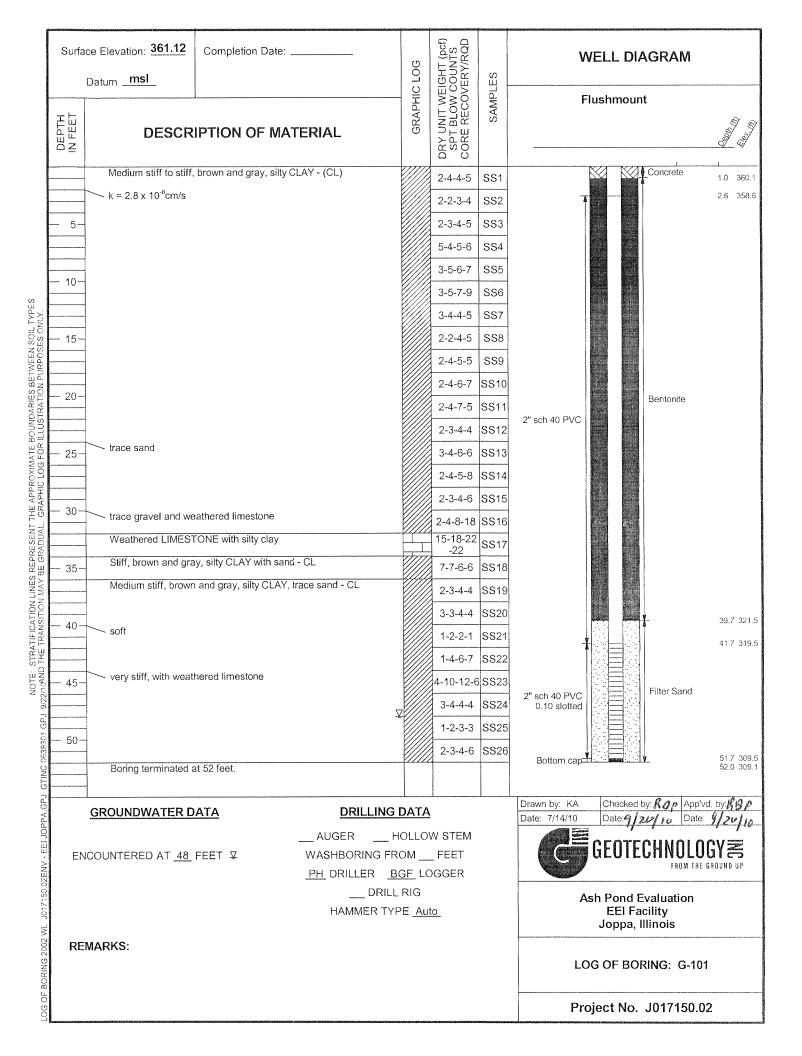
ATTACHMENT 3 – WELL CONSTRUCTION DIAGRAMS AND DRILLING LOGS

WE	CLIENT Sit Location Projec DATES	F: N e: Jo n: Jo t: 15 S: St Fir R: Pa	tart: 8/14 hish: 8/14 artly cloud	sourc er Sta bis 4/20 4/20 ly, ca	xe Te ation 15 15	echnolo warm,	gy, Inc. mid-70s	CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¼" HSA, split spoon sampl FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill Eng/Geo: S. Keim		Kell ID: G01D Surface Elev: 361.50 ft. MSL Completion: 64.38 ft. BGS Station: 202,039.30N 831,716.11E			
	Recov / Total (in)		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadrangle: Township: 1		Ţ	= 54.00 - = 45.77 -	INFORMAT During Drilling 8/14/15		
Number	Recov % Rec	Type	Blows N - V RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks	
	0/60 <i>0%</i>	BD					2			و ، و ، و ، و ، و ، و ، و ، (//////////	360		
	0/60 <i>0%</i>	BD					6 8 10			ر لی لی ر لی	356		
	0/60 <i>0%</i>	BD					10	Blind drill - see G101 boring log		ر لاے	350		
NC	0/60 0%	BD	D installe	din	bore	hole	16 18 18 20			لی ل	346		

	CLIEN Sit Location Projec DATE	F: Na e: Jo n: Jo t: 15 S: St Fin	BOR atural Re ppa Powe ppa, Illin 5E0030 cart: 8/14 nish: 8/14 artly cloud	sour er St ois 4/20 4/20	ce Te ation 15 15	echnolo	gy, Inc.	CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¼" HSA, split spoon sampl FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill Eng/Geo: S. Keim		BOREHOLE I Well I Surface El Completio	HANSON D: G01D D: G01D ev: 361.50 ft. MSL on: 64.38 ft. BGS on: 202,039.30N 831,716.11E				
5	SAMPL	E	Т	EST		-	TOPOGRAPI	HC MAP INFORMATION:		ATER LEVEL INFORMATION:					
ber	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadrangl Township: Section 14,	15S;3E Tier 15S; Range 3E	⊻ = 45.7 ⊻ =						
Number	Reco % Re	Type	Blow N - V RQI	Mois	Dry]	Qu (1 Failu		Lithologic Description	Boreh Deta		Remarks				
	0/60 0%	BD					22		, , , , , , , , , , , , , , , , , , ,						
	0/60 0%	BD					28	Blind drill - see G101 boring log [Continued from previous page]	, , , , , , , , , , , , , , , , , , ,	- 334					
	0/60 0%	BD					30 32 34 36 38 40		, , , , , , , , , , , , , , , , , , ,						
NC	0/60 0%	BD G01	D installe	ed in	bore	hole.	38		<u>, , , , , , , , , , , , , , , , , , , </u>	324					
											Page 2 of 4				

WE	ELI CLIENT Site Location Projec DATES ATHEF	er BOREHOLE ID: G01D Well ID: G01D Surface Elev: 361.50 ft. MSL Completion: 64.38 ft. BGS Station: 202,039.30N 831,716.11E										
5		E	Т	EST			TOPOGR	APHIC MAP INFORMATION:	WATER LEVEL INFORMATION:			
er	Recov / Total (in) % Recovery		/ 6 in lue	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadr Towns Section	angle: Joppa hip: 15S;3E 1 14, Tier 15S; Range 3E	Ψ = 54.00 - During Drilling Ψ = 45.77 - 8/14/15 $\overline{\Psi}$ =			
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moistu	Dry Do		Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks			
	0/48 <i>0%</i>	BD					42	Blind drill - see G101 boring log [Continued from previous page]	320			
9A	24/24 100%	ss	5-13 6-5 N=19	13		1.00		Strong brown (7.5YR5/6), moist, medium dense, silty, v fine- to coarse-grained SAND with trace small gravel.				
9B		_		20			⊻ 46					
10A	22/24 92%	ss	2-3 3-4 N=6	20		1.50	48	Gray (10YR6/1) with 40% yellowish brown (10YR5/6 mottles, moist, stiff, SILT, few to little clay, trace fine- medium-grained sand.				
11A	24/24 100%	ss	1-2 3-3 N=5	21		0.80			312			
12A	19/24 79%	ss	1-1 2-4 N=3	20		0.50	50	Gray (10YR6/1) with 30% yellowish brown (10YR5/6 mottles, moist, medium, silty CLAY, few fine- to medium-grained sand.	5) 310			
13A	24/24 100%	ss	woh-1 2-3 N=3	19		0.50	52 ⊻ 54	Gray (10YR6/1) with 25% yellowish brown (10YR5/t mottles, moist, medium, CLAY with few very fine- to fine-grained sand and trace silt.				
14A	19/24 79%	ss	14-18 20-24 N=38	14			56	Light yellowish brown (10YR6/4), moist, dense, silty, ve fine- to fine-grained SAND.	ery 306			
15A	16/24 67%	ss	woh-wol 5-13	^h 23			58	Light yellowish brown (10YR6/4), wet, dense, silty, ve fine- to fine-grained SAND.	ту 304			
16A	20/24 83%	ss	4-21 25-21 N=46	18			60	Gray (10YR6/1), wet, dense, silty, very fine- to coarse-grained SAND.				





FIELD BORRING LOG CONTRACTOR: Bulldog Drilling, Inc. Site: Joppa Power Station Rig mfg/model: CME-750 ATV Drill BOREHOLE ID: G02D Location: Joppa, Illinois Drilling Method: 4¼" HSA, split spoon sampler Well ID: G02D Project: 15E0030 Surface Elev: 360.82 DATES: Start: 8/12/2015 FIELD STAFF: Driller: J. Dittmaier Completion: 72.36 Finish: 8/13/2015 Helper: M. Hill Station: 202,11 WEATHER: Sunny, warm, calm, lo-80s Eng/Geo: S. Keim 832,8											: G02D : G02D : 360.82 ft. MSL : 72.36 ft. BGS	
S	SAMPLETESTINGTOPOGRAPHIC MAP INFORMATION:WATER LEVEL INFORMATION: $\widehat{\Xi}$ \widehat{T} \widehat{T} Ouadrangle: Joppa $\Psi = 43.00$ - During Drilling											
er	Recov / Total (in) % Recovery Type		<i>Blows / 6 in</i> N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Towns	angle: Joppa hip: 15S;3E 14, Tier 15S; Range 3E	₹ ₹ ∑	=	During Drillin	g
Number	Recov % Rec	Type	Blows N - V RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	ss	1-1 2-3 N=3	24		2.00	2 4 8 8	Grayish brown (10YR5/2), moist, soft, SILT with few c trace very fine- to medium-grained sand, wood fragmer and roots. Brown (10YR5/3) with 40% yellowish brown (10YR5	nts,	*/////////////////////////////////////	360	
2A	24/24 100%	ss	1-3 5-5 N=8	23		2.50	4	mottles, moist, stiff, SILT with few clay, trace very fine-grained sand and roots.		د م د م د م د م د م د م د م د م د م د م	358	
3A	24/24 100%	ss	1-3 5-5 N=8	20		2.00	6	Brown (10YR5/3) with 40% yellowish brown (10YR5 and 10% gray (10YR6/1) mottles, moist, stiff, SILT w few clay, trace very fine-grained sand.	5/6) rith	دے دے دے دے دے دے دے دے دے	356	
4A	19/24 79%	ss	1-4 4-6 N=8	21		2.30	8			- د د د د د د . د د د د د د د . د د د د د	354	
5A	24/24 100%	ss	1-4 5-6 N=9	20		3.00	10			ے لاے لاے لاے لا	352	
6A	20/24 83%	ss	1-2 5-6 N=7	20		2.50	12	Gray (10YR6/1) with 30% yellowish brown (10YR5/ mottles, moist, very stiff, SILT with few clay and trace v fine- to fine-grained sand.	/6) very	دے دے دے دے ہے۔ دے دے دے دے د	350	
7A	22/24 92%	ss	1-5 7-11 N=12	18		3.00	14			و ہے و ہے و ہے ہے۔ و ہے و ہے و ہے و ہے ا		
8A	19/24 79%	ss	2-9 13-18 N=22	15		4.50	10 12 12 14 16 18 18 18 18 10 10 10 10 10 10 10 10 10 10	Yellowish brown (10YR5/8) with 35% grayish brow (10YR5/2) mottles, moist, hard, SILT with few clay a trace very fine- to medium-grained sand.	n nd	و و و و و و	346	
9A	24/24 100%	ss	4-8 11-15 N=19	17		3.00	18	Gray (10YR6/1) with 30% yellowish brown (10YR5/ mottles, moist, very stiff, SILT with few clay and trace v	(6) very	وي وي وي وي وي وي وي وي وي وي وي وي		
10A	18/24 75%	ss	1-7 10-14 N=17	16		3.50	20	fine- to fine-grained sand.		2222		
NO)TE(S):	G02	D installe	 ed in	bore	hole.	20				F	D 1

			BOR atural Re					CONTRACTOR: Bulldog Drilling, Inc.		<	S H	ANSON
Site: Joppa Power StationRig mfg/model: CME-750 ATV DrillBOREHOLE ID: G02DLocation: Joppa, IllinoisDrilling Method: 4¼" HSA, split spoon samplerWell ID: G02D												
WE	Project: 15E0030 Surface Elev: 360 DATES: Start: 8/12/2015 FIELD STAFF: Driller: J. Dittmaier 72 Finish: 8/13/2015 Helper: M. Hill Station: 20 WEATHER: Sunny, warm, calm, lo-80s Eng/Geo: S. Keim 82 SAMPLE TESTING 82											
5	SAMPLETESTINGTOPOGRAPHIC MAP INFORMATION:WATER LEVEL INFORMATION: $\widehat{\underline{\exists}}$ $\widehat{\underline{a}}$ $\widehat{\underline{f}}$ \widehat											
	tal (in		ı	(0)	Dry Den. (lb/ft^3)	o (tsf) e	-	rangle: Joppa ship: 158;3E	_	= 43.00 -	During Drillin	ng
er	- / To		Blows / 6 in N - Value RQD	Moisture (%)		$(1)_{\rm QI} Q_{\rm I}$		n 14, Tier 15S; Range 3E		. =		
Number	Recov / Total (% Recovery	Type				Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks
11A	21/24 88%	ss	2-8 12-17 N=20	15		4.00	22			نے قے قے قے قے نے قے قے قے قے		
12A	22/24 92%	ss	4-9 12-13 N=21	16		4.00	24	Gray (10YR6/1) with 30% yellowish brown (10YR5/0		ي ٿي ٿي ٿي ٿي ٿي ي ٿي ٿي ٿي ٿي ٿ	338 	
13A	24/24 100%	ss	3-8 11-11 N=19	17		3.00	24	mottles, moist, very stiff, SILT with few clay and trace very fine- to fine-grained sand. [Continued from previous page]	rery	نے فے فے فے فے ف		
14A	20/24 <i>83%</i>	ss	2-6 8-9 N=14	16		2.50	28-			لي لي لي لي لي ا لي لي لي لي لي ا		
15A	22/24 92%	ss	1-5 7-8 N=12	19		3.30	30	Gray (10YR6/1) with 30% yellowish brown (10YR5/0 mottles, moist, very stiff, SILT with little clay and trac very fine- to fine-grained sand.		، و ہے و ہے و ہے و ہے ، و ہے و ہے و ہے و ہے ، و ہے و ہے و ہے و ہے		
16A	24/24 100%	ss	1-6 8-9 N=14	17		2.30	32	Yellowish brown (10YR5/4) with 10% gray (10YR6/ mottles, moist, very stiff, SILT with few clay and very fi to fine-grained sand, trace small gravel.	1) ine-	دے تے تے تے تے ت	330	
17A	23/23 100%	ss V	2-7 40-50/5' N=47	16		2.80	34	Strong brown (7.5YR4/6), moist, very stiff, SILT with livery fine- to coarse-grained sand and small to large grav		و و و و و م و م و م و م و م و م و م و م	328	
18A	18/24 75%	ss	8-25 43-19 N=68	11				Strong brown (7.5YR4/6), moist, very dense, silty, ver fine- to coarse-grained SAND with little small to large gravel.			326	
19A	24/24 100%	ss	1-3 4-30 N=7	22		0.80	36	Yellowish brown (10YR5/4) with 10% gray (10YR6/ mottles, moist, medium, silty CLAY with few fine- to medium-grained sand, trace gravel.	1)		- 324	
20A	12/16 75%	ss	6-35 50/4"	13			38	Strong brown (7.5YR4/6) moist, hard, SILT with little v fine- to coarse-grained sand and little small to large grav		ے لے لے لے لے ل	- 322	
NC)TE(S):	G02	 D installe	 ed in	bore	hole.	40 <u>⊣</u>			4 4		Page 2 of 4

(CLIENT Sit Location Projec DATES	F: Na e: Jo n: Jo et: 15 S: St Fin	art: 8/12 nish: 8/12	sourd er Sta ois 2/20 3/20	ce Te ation 15 15	chnolo		CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¼" HSA, split spoon sampl FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill	er	во	DREHOLE ID Well ID Surface Elev Completion	: G02D : 360.82 ft. MSL : 72.36 ft. BGS : 202,137.08N
	ATHE		inny, war T		aim, FINC		тороср	Eng/Geo: S. Keim	WAT			832,842.99E
Number	Recov / Total (in) % Recovery	ē	<i>Blows / 6 in</i> N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadra Townsł	APHIC MAP INFORMATION: ingle: Joppa iip: 15S;3E 14, Tier 15S; Range 3E Lithologic	Ā		L INFORMAT - During Drillin e Elevation	
Nur		-			Dry	Qu Fail	ft. BGS	Description		Detail	ft. MSL	Remarks
21A	9/9 100%	X ss	39-50/3'	11			42	Strong brown (7.5YR4/6) moist, hard, SILT with little v fine- to coarse-grained sand and little small to large gra [Continued from previous page]	very vel.	د و و و و و و و و و و د و و و و و د و و و و	- 320	
22A	8/24 <i>33%</i>	ss	5-14 17-8 N=31	15			⊻ 44 <u>–</u>	Brownish yellow (10YR6/6), moist, dense, silty, fine-grained SAND with trace medium- to coarse-grain sand.	ned			
23A	24/24 100%	ss	1-2 3-4 N=5	22		0.50	46	Brownish yellow (10YR6/6), moist, medium, silty CL with few very fine- to medium-grained sand.	AY 		- 316	
24A	18/24 75%	ss	woh-woh 4-7	18		0.50	48	Brownish yellow (10YR6/6), moist, soft, CLAY with so very fine- to medium-grained sand and trace coarse-grai sand and trace small gravel.	ome ined		- 314	
25A	20/24 83%	ss	1-2 6-5 N=8	18		0.80	50	Brownish yellow (10YR6/6), moist, medium, CLAY w some very fine- to medium-grained sand and trace coarse-grained sand and trace small gravel.	vith		-312	
26A	17/24 71%	ss	2-5 5-8 N=10	23		2.50	52	Gray (10YR6/1) with 40% yellowish brown (10YR5/	6)		- 310	
27A	22/24 92%	ss	1-3 3-5 N=6	18		1.50	54	mottles, moist, stiff, silty CLAY with few fine-grained s			- 308	
28A	24/24 100%	ss	woh-3 3-4 N=6	17		1.00	56	Gray (10YR6/1), moist, medium, CLAY with some fine-grained sand and few silt.	:		- 306	
29A	24/24 100%	ss	woh-4 5-5 N=9	22		3.00	58	Gray (10YR6/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, stiff, SILT with few clay, tra	ace	تے قے قے قے قے ا	304	
30A	20/24 83%	ss	2-4 6-6 N=10	19		2.00	58	fine-grained sand.			302	
NO	TE(S):	G02	D installe	ad in	bore	nole.						Page 3 of 4

I	Sit Location Projec DATES	e: Jo n: Jo t: 15 S: St Fin	atural Re ppa Pow ppa, Illin 5E0030 cart: 8/1 nish: 8/1 nny, wan	er Sta iois 2/20 3/20	ation 15 15		gy, Inc.	CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¼" HSA, split spoon sampler FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill Eng/Geo: S. Keim		REHOLE ID Well ID Surface Elev Completion	
	Recov / Total (in) W				Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadr Towns	APHIC MAP INFORMATION: angle: Joppa hip: 158;3E 14, Tier 158; Range 3E	WATER LEVEL $\Psi = 43.00 - $ $\Psi = $ $\overline{\Psi} = $		
Number	Recov % Rec	Type	<i>Blows / 6 in</i> N - Value RQD	Moisti	Dry D	Qu (ts Failure	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
	19/24 79%	ss	woh-1 4-14 N=5	20			62	Gray (10YR6/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, medium, SILT with little clay and little very fine- to medium-grained sand.	y	300	
2A	16/24 67%	ss	3-14 18-25 N=32	20				Gray (10YR6/1), wet, dense, very fine- to coarse-grained SAND with some silt.	d	298	
3A	14/24 58%	ss	5-18 37-33 N=55	18				Gray (10YR6/1), wet, dense, very fine- to coarse-grained SAND with some silt and trace small to large gravel.	d	296	
1A	16/24 67%	ss	18-29 35-21 N=64	13			66	Brownish yellow (10YR6/6), wet, very dense, silty, very fine- to coarse-grained SAND and small to large GRAVE	L. 0	294	
5A	14/24 58%	ss	20-24 30-30 N=54	16						292	
6A	16/24 67%	ss	12-19 37-28 N=56	14			70	Gray (10YR6/1), wet, very dense, silty, very fine- to coarse-grained SAND with trace small to large gravel.			
	0/4 <i>0%</i>	BD					72 -	End of boring = 72.36 feet			
								End of boring - 72.50 feet			

) WE	CLIENT Sit Location Projec DATES	F: Na e: Jo n: Jo n: Jo t: 15 S: St S: St Fin R: Pa	BOR atural Re oppa Powe oppa, Illin 5E0030 tart: 8/1 aish: 8/1 artly cloud	sour er Sta ois 7/20 8/20	ce Te ation 15 15	echnolo	gy, Inc.	CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¼" HSA, split spoon sampl FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill Eng/Geo: R. Hasenyager	er		REHOLE ID Well ID Surface Elev Completion	
S	GAMPL	E	Т	EST			TOPOGRAPH	IC MAP INFORMATION:			INFORMA During Drillin	
	Total (i 'ery		5 in Ie	(%) 0	. (lb/ft ³	$\frac{Qp}{V}$ (ts.	Quadrangle: Township: 1 Section 14-7			= 34.91 -		Ig
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks
	0/60 0% 0/60 0%	BD BD BD					10 10 11 12 14 10 11 12 14 16 18 10 11 12 14 16 18 10	Blind drill - see G151 boring log		تو و و و و و و و و و و و و و و و و و و		
												Page 1 of 3

F	[EL]		BOR	I	NG	G L	OG			6	E	ANSON
]	Site Location Projec DATES	e: Jo n: Jo t: 15 S: St Fin	atural Re ppa Powe ppa, Illin 5E0030 cart: 8/1 nish: 8/1 artly cloud	er Sta iois 7/20 8/20	ation 15 15	1		CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4 ¹ /4" HSA, split spoon sample FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill Eng/Geo: R. Hasenyager	er		REHOLE ID Well ID Surface Elev Completion	: G51D
	SAMPL				TINC			IC MAP INFORMATION:	WATEI		INFORMA	
L	Recov / Total (in) % Recovery		<i>6 in</i> ue	re (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadrangle Township: Section 14, 7	: Joppa	▼ =	= 39.50 - 1 = 34.91 -	During Drillin	
Number	Recov	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry De	Qu (tsf Failure	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks
	0/60 0% 0/60 0% 0/60 0%	BD BD BD BD					$ \begin{array}{c} 11. \ BO3 \\ 22 \\ 24 \\ 26 \\ 28 \\ 30 \\ 32 \\ 32 \\ 34 \\ \hline \mathbf{Y} \\ 36 \\ 38 \\ \hline \mathbf{Y} \\ 36 \\ \hline \mathbf{Y} \\ 38 \\ \hline \mathbf{Y} \\ 36 \\ \hline \mathbf{Y} \\ 38 \\ \hline \mathbf{Y} \\ \mathbf{Y} $	Blind drill - see G151 boring log [Continued from previous page]		وہ و		
9A	24/24 100%	SS	6-7 4-4 N=11	22		1.50	m	Yellowish brown (10YR5/8) with 20% gray (10YR5/1 ottles, moist, medium, CLAY with few silt and little ve fine- to fine-grained sand.	l) ery		320	
NO)TE(S):	G51	D installe	ed in	bore	ehole.						Page 2 of 3

	CLIENT Site Location Projec DATES	F: N e: Jo n: Jo t: 15 S: St Fir	BOR atural Re oppa Powe oppa, Illin 5E0030 tart: 8/1 aish: 8/1 artly cloud	sourd er Sta ois 7/20 8/20	ce Te ation 15 15	echnolo	gy, Inc.	CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¼" HSA, split spoon sampler FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill Eng/Geo: R. Hasenyager			REHOLE II Well II Surface Ele Completio	HANSON D: G51D D: G51D v: 361.10 ft. MSL n: 59.90 ft. BGS n: 200,430.10N 832,151.51E
	Recov / Total (in) W	E			LINC (lb/ft ³) II	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadr Towns	APHIC MAP INFORMATION: angle: Joppa hip: 15S;3E 114, Tier 15S; Range 3E	▼ =	39.50 - 34.91 -	INFORMA During Drill 8/18/15	
Number	Recov / Toti % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure	Depth ft. BGS	Lithologic Description	-	Borehole Detail	Elevation ft. MSL	Remarks
0A	24/24 100%	ss	1-2 4-5 N=6	20		1.50	44	Yellowish brown (10YR5/8) with 20% gray (10YR5/1) mottles, moist, medium, CLAY with few silt and little ver fine- to fine-grained sand. [Continued from previous page]	y		-318	
1A	24/24 100%	ss	3-4 4-4 N=8	16		0.25	44 46	Yellowish brown (10YR5/8) with 50% gray (10YR6/1) mottles, moist, soft, very fine- to medium-grained SAND with some silt and little clay.			316	
2A	24/24 100%	ss	5-7 10-11 N=17	13		1.50		Yellowish brown (10YR5/8) with 50% gray (10YR6/1) mottles, moist, stiff, very fine- to medium-grained SAND with some silt, little clay, and trace small gravel			314	
3A	20/24 <i>83%</i>	ss	5-12 14-15 N=26	15		0.25		Yellowish brown (10YR5/8) with 20% gray (10YR6/1) mottles, wet, loose, very fine- to medium-grained SAND with trace silt, trace clay and trace small gravel.			312	
4A	23/24 96%	ss	5-15 19-21 N=34	15		1.25	48				310	
5A	22/24 92%	ss	3-12 14-7 N=26	15		1.50	54				308	
6A	24/24 100%	ss	<i>1-5</i> <i>6-11</i> N=11	17		0.00	54 56 58	Strong brown (7.5YR5/8), wet, loose, very fine- to medium-grained SAND with trace silt, trace clay, and trac small gravel.	æ		306	
7A	24/24 100%	ss	2-7 8-11 N=15	19		0.00						Duillod most in d
	0/23 <i>0%</i>	BD					58				302	Drilled past end o sample interval
I	L]	I	I	I	I		End of boring = 59.9 feet		K) (111 111 - 121)	<u>_</u>	I
NO	TE(S):	G51	D installe	ed in	bore	hole.						
												Page 3 o

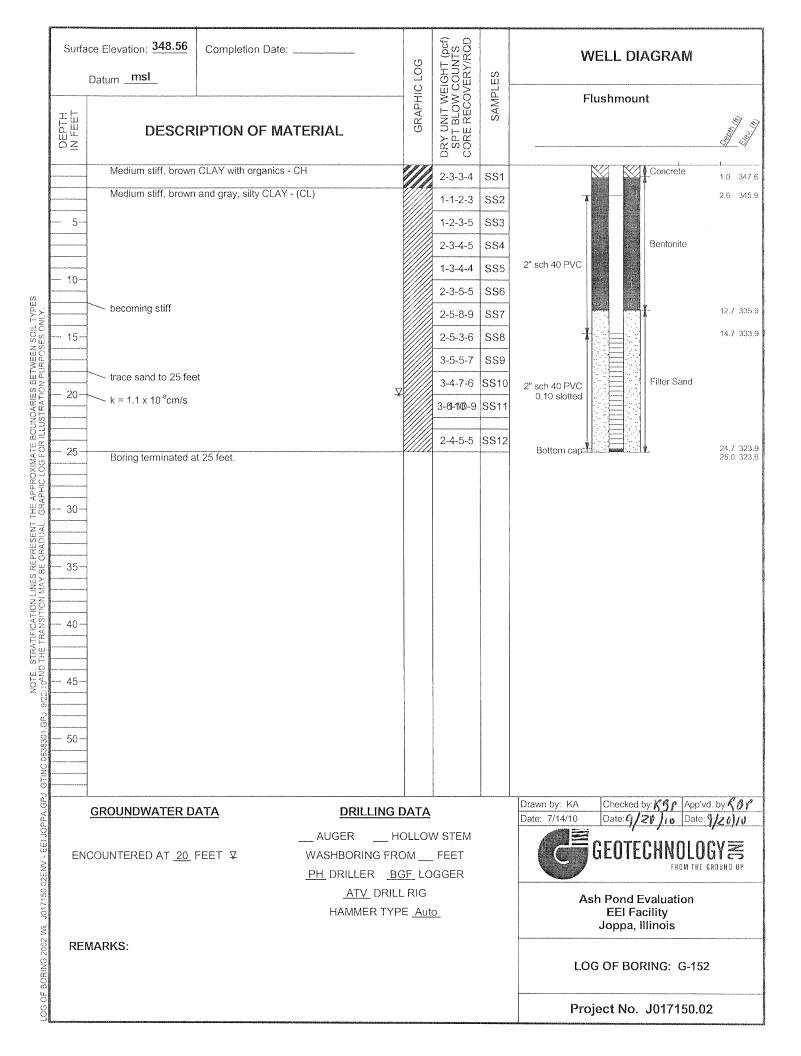
	ce Elevation: <u>360.9</u> Datum <u>msl</u>	Completion Date: 6/19/10	POG	GHT (pcf) OUNTS ERY/RQD	ES	WELL DIAGRAM
DEPTH IN FEET		IPTION OF MATERIAL	GRAPHIC LOG	DRY UNIT WEIGHT (pcf) SPT BLOW COUNTS CORE RECOVERY/RQD	SAMPLES	Flushmount
- 5- - 10- - 15- - 20- - 25- - 30- - 30- - 35- - 35- 	✓ k = 1.6 x 10 ⁻⁸ cm/s Medium stiff to stiff, CL	brown and gray, silty CLAY - (CL) brown and gray, silty CLAY, trace sand - stiff, brown and gray, sandy CLAY with		3-4-4-5 2-3-3-5 3-3-4-4 1-2-3-5 3-5-5-6 2-4-5-6 2-3-4-5 3-4-6-7 3-4-6-6 3-5-7-8 3-5-7-9 2-3-5-6 3-5-7-6 3-5-7-6 3-5-7-6 3-5-7-6 3-5-8-8 4-7-9-4 4-4-6-7 2-4-3-5 5-5-7-14 13-13-10 -8 2-4-6-7	SS11 SS12 SS13 SS14 SS15 SS16 SS17 SS18 SS18	2" sch 40 PVC 2" sch 40 PVC 2" sch 40 PVC 0.10 slotted Bottom cap
ENCC	GROUNDWATER D	OTAUGER	HOLLO' FROM BGF_LC	W STEM _ FEET)GGER		Drawn by: KA Checked by: Ry App'vd. by: R & Date: 7/14/10 Date: 9/20/10 Date: 9/20/10 GEOTECHNOLOGY S FROM THE GROUND UP Ash Pond Evaluation EEI Facility Joppa, Illinois LOG OF BORING: G-151 Project No. J017150.02

F	EL	DI	BOR	IN	IG	L	OG			6	<a>H	ANSON
I	Sit Locatio Projec DATE	e: Jo n: Jo xt: 15 S: St Fir	atural Res ppa Powe ppa, Illino 5E0030 cart: 8/18 nish: 8/19 vercast, hu	er Sta ois 8/20 9/20	ation 15 15		gy, Inc.	CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¼" HSA, split spoon samp FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill Eng/Geo: R. Hasenyager	oler		REHOLE ID Well ID Surface Elev Completion	: G52D
	SAMPL				'INC		TOPOCDADU		WATE			
	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadrangle Township: Section 14, 7	15S;3E Гіег 15S; Range 3E		= 28.45 - 1 = =	INFORMAT During Drillin	
Number	Reco % Re	Type	Blow N - V RQD	Mois	Dry I	Qu (t Failu	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks
	0/60 0% 0/60 0%	BD					2			فر ف	344 342 340 338	
	0/60 <i>0%</i>	BD						Blind drill - see G152 boring log		ن فرد	336	
	0/60 <i>0%</i>	BD					12			ر قرم قرم قرم قرم قرم قرم قرم قرم قرم قر	330	
NO	TE(S):	G52	D installe	d in	bore	hole.				••• -•	·	Page 1 of 4

C I	CLIENT Site Location Projec DATES ATHEF	F: Na e: Jo n: Jo t: 15 S: St Fin R: O	art: 8/13 nish: 8/1 vercast, h	sourc er Sta ois 8/20 9/20 iumic	te Te ation 15 15 1, mid	BO	REHOLE ID Well ID Surface Elev Completion	: G52D : 345.88 ft. MSL			
S	AMPLI (ii)	E	Т	EST			TOPOGRAPH Quadrangle	IC MAP INFORMATION: :: Joppa	WATER LEVEL $\underline{\Psi} = 28.45$ -	L INFORMAT	
;	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Township: Section 14, 7	15S;3E Гіег 15S; Range 3E	$\underline{\Psi} = $ $\underline{\nabla} =$		
	Recov % Rec	Type	Blows N - V RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
	0/60 0% 0%	BD					22 24 26 28 30		<u>وہ ہے ہے</u>	324 322 320 318	
	0/60 <i>0%</i>	BD						Blind drill - see G152 boring log [Continued from previous page]	<u> </u>	314	
	0/60 <i>0%</i>	BD					32 34 36 38 40 42		<u> </u>	310	
	0/24 <i>0%</i>	BD							<u>,,,,,</u>	304	

(CLIENT Site Location Projec	F: Na e: Jo n: Jo t: 15		souro er Sta ois	ce Te ation	chnolo		CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4 ¹ /4" HSA, split spoon sample	Surface Elev: 345.88 ft. MSL
WE		Fin	tart: 8/13 nish: 8/1 vercast, h	9/20	15	d-70s		FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill Eng/Geo: R. Hasenyager	Completion: 80.01 ft. BGS Station: 198,098.93N 832,927.89E
S	AMPL	E	Т	EST	ING			PHIC MAP INFORMATION:	WATER LEVEL INFORMATION:
er	Recov / Total (in) % Recovery		/ 6 in lue	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Townshi	ngle: Joppa ip: 15S;3E 14, Tier 15S; Range 3E	$\mathbf{\Psi} = 28.45$ - During Drilling $\mathbf{\Psi} = \mathbf{\Psi} = \mathbf{\Psi} = \mathbf{\Psi}$
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moist	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks
10A	24/24 100%	ss	3-3 5-6 N=8	22		3.75	44	Gray (10YR5/1) with 40% yellowish brown (10YR5/8	302
11A	24/24 100%	ss	N=7 21 3. 3-4 20 3.		3.75	46	mottles, moist, medium, CLAY with some silt and trace very fine-grained sand.	300	
12A	24/24	ss	3-4 4-7	20		3.50	40		
12B	100%		N=8	23			48	Gray (10YR6/1), moist, medium, SILT with some very fine-grained sand and trace clay.	298
13A	24/24 100%	ss	2-4 5-6 N=9	23			50	Gray (10YR5/1) with 30% yellowish brown (10YR5/8 mottles, moist, medium, CLAY with some silt and trace very fine-grained sand.	
14A	24/24 100%	ss	5-7 8-10 N=15	20		4.00	50	Gray (10YR6/1) with 15% yellowish brown (10YR5/8 mottles, moist, stiff, CLAY with some silt and trace ver fine-grained sand.	
15A	24/24 100%	ss	1-4 4-5 N=8	24		2.50	³² Ξ	Gray (10YR6/1) with 10% yellowish brown (10YR5/8 mottles, moist, medium, SILT with few very fine-graine sand and little clay.) d
16A	24/24 100%	ss	2-3 7-7	24		2.50	54	Gray (10YR6/1), moist, soft, CLAY with some silt and trace very fine-grained sand. Gray (10YR6/1) with 10% yellowish brown (10YR5/8 mottles, moist, soft, SILT with little clay and very	
16B	10070		N=10	20			56	fine-grained sand. Gray (10YR6/1), moist, soft, CLAY with some silt and trace very fine-grained sand.	290
17A	24/24 100%	ss	4-4 7-9 N=11	21		3.75		Gray (10YR6/1), moist, medium, SILT and very fine-grained SAND with few clay.	
18A	24/24 100%	ss	2-6 7-7 N=13	24		3.25	54 56 58 60	Gray (10YR6/1), moist, medium, interbedded (0.1-0.2' SILT and very fine-grained SAND with few clay and CLAY with some silt and trace very fine-grained sand.	
19A	22/24 92%	ss	3-3 4-4 N=7	20		2.25			
.	24/24	$\overline{\langle}$	2-3				62	Gray (10YR6/1), moist, medium, SILT with very fine-grained sand and few clay.	
NO	TE(S):	G52	D installe	ed in	bore	hole.			

I	Sit Location Projec DATES	e: Jo n: Jo t: 1: 5: Si Fin	oppa Pow oppa, Illin	er Sta ois 8/20 9/20	ation 15 15	ology, Inc.)s	CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¼" HSA, split spoon samp FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill Eng/Geo: R. Hasenyager	ler		REHOLE I Well I Surface Ele Completio	HANSON D: G52D D: G52D W: 345.88 ft. MSI n: 80.01 ft. BGS n: 198,098.93N 832,927.89E
	Recov / Total (in) AW	E		Moisture (%)	Dry Den. (lb/ft ³) S Qu (tsf) <i>Qp</i> (tsf)	TOPOC Qua add Tow Secti	RAPHIC MAP INFORMATION: Irangle: Joppa nship: 15S;3E on 14, Tier 15S; Range 3E	WATER LI $\underline{\Psi} = 28$ $\underline{\Psi} =$ $\underline{\nabla} =$		INFORM A During Drill	
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moist	Dry D Qu (ts	Depth ft. BGS	Lithologic Description		ehole etail	Elevation ft. MSL	Remarks
0A	100% 24/24 100%	ss	5-6 N=8 4-4 6-7	23	2.					282	
A	24/24 100%	ss	N=10 3-4 8-9 N=12	23		25 64 25 66 68 50	Gray (10YR6/1), moist, medium, SILT with very fine-grained sand and few clay. [Continued from previous page]		2 (2 (2 ()	280	
3A	24/24 100%	ss	2-7 6-6 N=13	22	2.	50				278	
A	24/24 100%	ss	wor-6 9-9 N=15	22	2.	50	Gray (10YR6/1) with 20% yellowish brown (10YR5/ mottles, moist, stiff, SILT and very fine-grained SAND few clay.	8) with		 274	
A	24/24 100%	ss	wor-3 5-7 N=8	22	2.	25	Gray (10YR6/1), wet, dense, SILT and very fine-grain SAND with trace clay.				
AB	24/24 100%	ss	6-7 8-9 N=15	17 22	3	25	SAND with trace sitt.	ned [1]			
A	24/24 100%	ss		22		25 76 75 78 25	Yellowish brown (10YR5/8) with 10% gray (10YR5/ mottles, moist, stiff, CLAY with some silt and trace ve fine-grained sand.	(1) ry		- 270 	Rods dropped, no blows
A	22/24 92%	ss	woh-3 6-7 N=9	27	3.	25	End of boring = 80.01 feet			266	





acilit	y/Projec	et Nan	ne		License/	Permit	Monitor	ing Ni	umber		Boring		ge 1	of	3
			tation	(EEI)		. emit					-	G15			
Boring	g Drilleo	d By:	Name c	of crew chief (first, last) and Firm	Date Dri	lling S	tarted		Da	te Drill				Drill	ing Metho
	tt Cooj		Ŧ			1 /0 (10010				1 /2 0 /2	010			ollow ster
Bul	Idog L	rillir	ng, Inc	Common Well Name	Final Sta		3/2013	1	Surfac	e Eleva	$\frac{1/30/2}{\text{tion}}$	2013	Bo	au	ger Diameter
				G152B			NAVE			5.2 Fee		VD88			inches
	Grid Or	rigin	— `	stimated:) or Boring Location	1		0	· · · /	"		Grid Lo		/ _		
State	Plane			nois East Zone N, E $S/C/\mathbb{N}$.t		,				N			
acilit	1/4 v ID	of]	1/4 of Section , T N, R County	Long Long	g	Civil To	wn/Ci	ity/ or		4.58 Fe	et∐ S	8329	31.61	Feet 🗌 V
ueini	y ID				Illinois		Joppa		ity/ OI	v muge					
Sar											erties				
	-		t.	Soil/Rock Description					PID 10.6 eV Lamp						
e	Att. ed (j	Blow Counts	Depth In Feet	And Geologic Origin For				-	6 eV	ssive (tsf	0		2		nts
Number and Typ	gth 4 over	∑ ≈	th Ir	Each Major Unit		CS	Graphic Log	l gran	10.6	npren	sture	it	ticit	9	RQD/ Comments
Number and Type	Length Att. & Recovered (in)	Blov	Dep			U S	Grap Log	Well Diagram] []	Compressive Strength (tsf)	Moisture Content	Liquid Limit	Plasticity Index	P 200	RQD/ Comm
1	42 19		-	0 - 0.75' SILTY CLAY CL, disturbed with gra tree limbs, wood from clearing activities; dark	avel, brown	CL									PP 2 - 2.
				wet.				i							
			-	0.75 - 4.5' SILTY CLAY CL, light yellowish b (10YR 6/4), high plasticity, very soft to soft, s	ilty clay										
			-2	with organics (roots), soft, high plasticity, ligh yellowish brown (10YR 6/4), moist.	t										
			Ē												
			F,			CL									
			-3												
2	60 56		-4							3.25					
			-4	4' Silty Clay grading to a Clayey Silt, low plast stiff, light gray (10YR 7/1), with 50% reddish l	sticity,										
			-5	\mottling, moist.	/										
			= 3	4.5 - 12.5' SILT: ML, non plastic, stiff, very p brown (10YR 8/2), dry.	ale					3					
			F_												
			6							3					
			- 7												
			E′	7.3' soil horizon with small rootlets, 50% redo	dich					2					
			-8	brown mottling.											
			Ē	7.5' very pale brown (10YR 8/2), non plastic, dry.	stiff,	ML				1.75					
3	60 58		-9							4					
	50		Ē	9' silt with clay, very stiff, non plastic, very pa brown (10YR 7/3) with 10-25% reddish brown	ale										
			-	mottling.	1										
			-10							3.5					
			È.,,												
			11 							2					
			= 12												
		5, 4L - 4	+12	provide an this forms is true as 1	of m 1		daa		1						<u> </u>
here ignat	-	y that	ine info	prmation on this form is true and correct to the be	-		-	1					(2(2)	500 01	000
-511di				Firm Natu 23713	ral Reso				ikee V	VI 530'	72		(262) (262)		
				25715		uu L									GINT 2126



SOIL BORING LOG INFORMATION SUPPLEMENT

Boring Number G152B

Page 2 of 3 Sample Soil Properties PID 10.6 eV Lamp Length Att. & Recovered (in) Soil/Rock Description Compressive Strength (tsf) **Depth In Feet** Blow Counts And Geologic Origin For Comments Number and Type Moisture Plasticity Index Diagram Graphic $\boldsymbol{\mathcal{O}}$ Content Liquid Limit Each Major Unit USCS RQD/ P 200 Well Log 2 12.5 - 28.5' SILTY CLAY CL, medium to stiff, low to medium plasticity, light gray (10YR 7/1), with 50% 13 2 mottling, moist. 4 60 13.5' 10-50% reddish brown mottling. 60 E -14 2.5 ┝ 15 15' light gray (10YR 7/1). 2.25 -16 2.25 _ -17 3.25 F -18 18' yellowish brown (10YR 6/8) mottling, moist. PP 2.5 - 3.5 5 60 18.5' medium to stiff, medium to high plasticity, light 60 gray (10YR 7/1), with 10-25% reddish brown - 19 2.25 mottling, moist. -20 2.5 CL -21 1.75 ┝ 22 1.75 23 1.75 23.5' medium to stiff, high plasticity, light gray (10YR 7/1), with 25-50% reddish brown mottling, 6 60 60 24 2 moist. 25 1.5 ·26 1.5 27 2.5 28 2 28.5 - 29.5' CL. stopped sampling for NR 12 0 29 CL the day (1/28/13). 7 29.5 - 44.5' SILTY CLAY CL, stiff, medium to high 60 cleaned plasticity, gray with >75% light yellowish brown (10YR 6/4) mottling, moist. 44 30 2.75 hole with augers to 29.5 CL -31 resumed 3.25 sampling on 1/30/2013 31.5' 25-75% mottling. -32

NATURAL RESOURCE TECHNOLOGY

SOIL BORING LOG INFORMATION SUPPLEMENT

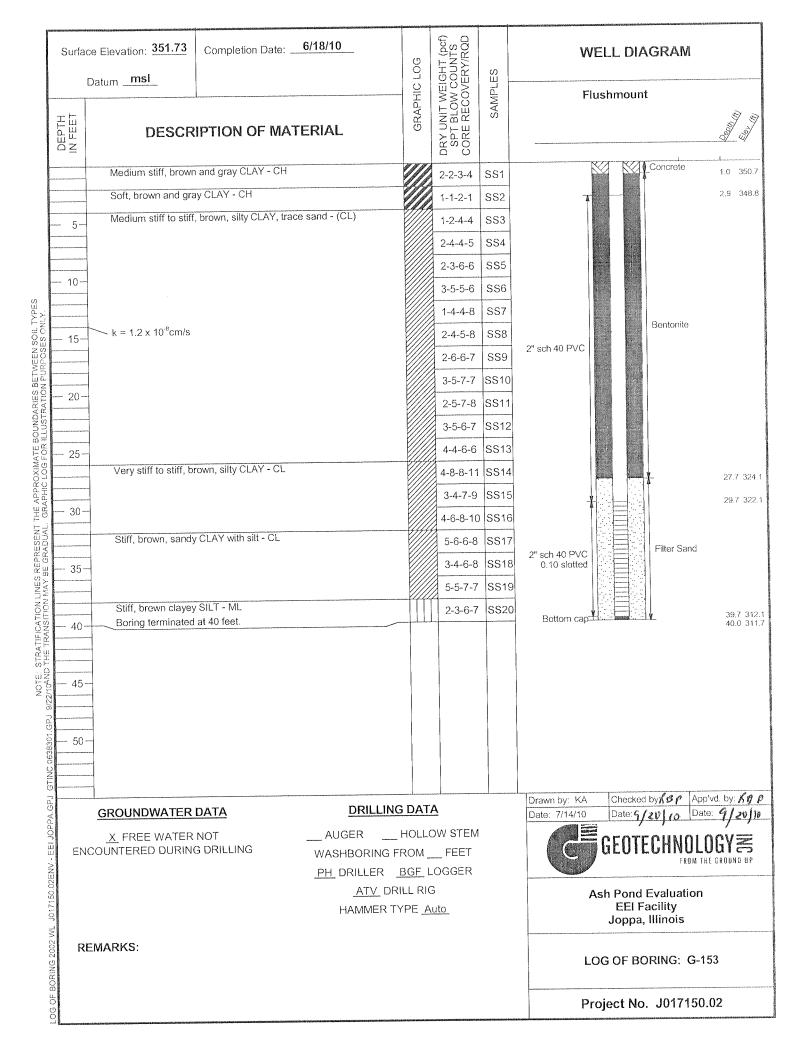
Boring Number G152B

3 Page of 3 Sample Soil Properties PID 10.6 eV Lamp Length Att. & Recovered (in) Soil/Rock Description Compressive Strength (tsf) Depth In Feet Blow Counts And Geologic Origin For Comments Number and Type Moisture Plasticity Index Diagram Graphic Content $\boldsymbol{\mathcal{O}}$ Liquid Limit Each Major Unit USCS RQD/ P 200 Well Log 29.5 - 44.5' SILTY CLAY CL, stiff, medium to high 3.5 plasticity, gray with >75% light yellowish brown (10YR 6/4) mottling, moist. *(continued)* 33 33' very stiff to hard, gray (10YR 6/1), with < 25% yellowish brown (10YR 6/8) mottling. 4.5 F - 34 2.5 8 34.5' stiff to very stiff, high plasticity. 60 57 F -35 3.5 -36 2.5 ⊨37 4.25 F -38 38' hard, < 10% mottling. 2.5 CL -39 PP is >4.5 39.5' very stiff to hard, high plasticity, gray (10YR 6/1) with < 10% yellowish brown (10YR 5/6) mottling, 9 60 55 40 3 moist. 41 3 42 3 43 4.5 -44 3.75 44.5' End of Boring.

WE	CLIEN Sit Locatio Projec DATE ATHEI	T: N te: Jo n: Jo ct: 1: S: St Fin R: St	BOR atural Res oppa Powe oppa, Illino 5E0030 tart: 8/20 hish: 8/21 unny, milc	sourc er Sta ois 0/20 1/20 1 mic	ce Te ation 15 15 d-60s	echnolo	gy, Inc.	CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¼" HSA, split spoon sampl FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill Eng/Geo: R. Hasenyager		REHOLE ID: Well ID: Surface Elev: Completion: Station:	G53D 352.16 ft. MSL 58.00 ft. BGS 200,075.16N 833,980.21E
	Recov / Total (in)	E		Moisture (%)	Dry Den. (lb/ft ³) Z	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadrangle Township:		= 43.45 - =	INFORMAT During Drilling	
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moistu	Dry D	Qu (ts Failur	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
	0/60 0%	BD					2		، في بالا///////////////////////////////////	352	
	0/60 0%	BD						Blind drill - see G153 boring log	ے کے لیے کے لیے لیے لیے لیے لیے لیے لیے ل ے لیے لیے لیے لیے لیے لیے لیے لیے لیے ل	344 	
	0/60 0%	BD					12		ے فے کے ۔ م لے فی فے فی فی فی فی فی فی فی فی ا	340	
NO	0/60 0% ВD 18- 18- 18- 18- 18- 18- 18- 19- 20- 19- 19- 19- 19- 19- 19- 19- 19- 19- 19					hole.	18-		وہ و وہ وہ	334	Page 1 of 3

Location: Joppa, Illinois Drilling Method: 4¼" HSA, split spoon sampler Project: 15E0030 FIELD STAFF: Driller: J. Dittmaier Finish: 8/21/2015 FIELD STAFF: Driller: J. Dittmaier WEATHER: Sunny, mild mid-60s Eng/Geo: R. Hasenyager											Well ID: G53D Surface Elev: 352.16 ft. MSL Completion: 58.00 ft. BGS Station: 200,075.16N 833,980.21E		
					1		PHIC MAP INFORMATION: agle: Joppa		ATER LEVEL INFORMATION: \mathbf{Y} = 43.45 - During Drilling				
er	% Recovery % Recovery % Recovery % Recovery NN - Value Blows / 6 in NN - Value RQD Moisture (%) Dry Den. (lb/ft ³) Dry Den. (lb/ft ³) Dry Den. (lb/ft ³)			Qu (tsf) <i>Qp</i> (tsf) Failure Type	Townshi		Ţ	$\mathbf{\underline{V}} = \mathbf{\underline{V}}$					
Number	A A <th>Depth ft. BGS</th> <th>Lithologic Description</th> <th></th> <th>Borehole Detail</th> <th>Elevation ft. MSL</th> <th>Remarks</th>				Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks			
	0/60 0% 0/60 0%	BD					22			ہے کی کے لیے کے لیے کے لیے لیے لیے لیے لیے لیے لیے لیے لیے لی			
	0/60 <i>0%</i>	BD					30 32 34 36 38 40	Blind drill - see G153 boring log [Continued from previous page]		لان			
	0/36 <i>0%</i>	BD					36			لي في في في في في في في في في في في في ف	316		
9A	24/24 100%	ss	3-3 5-5 N=8	19		1.25	38	Gray (10YR5/1) with 30% yellowish brown (10YR5/ mottles, moist, dense, very fine- to medium-grained SAI with some silt and little clay.	8) ND		314		
10A	21/24 88%	ss	3-6 7-8 N=13	17		1.50	40	Dark yellowish orange (10YR6/6) with 20% gray (10YR5/1) mottles, wet, dense, very fine- to medium-grained SAND with some silt, little clay, and to small gravel.			312		
NO	NOTE(S): G55D installed in borehole.												

) WE	FIELD BORRING LOGG CONTRACTOR: Bulldog Drilling, Inc. Image: Contractor is Bulldog Drilling, Inc. Site: Joppa Power Station Rig mfg/model: CME-750 ATV Drill BOREHOLE ID: G53D Location: Joppa, Illinois Drilling Method: 4¼" HSA, split spoon sampler Well ID: G53D Project: 15E0030 Surface Elev: 352.16 ft. MSL DATES: Start: 8/20/2015 FIELD STAFF: Driller: J. Dittmaier Completion: 58.00 ft. BGS Finish: 8/21/2015 Helper: M. Hill Station: 200,075.16N WEATHER: Sunny, mild mid-60s Eng/Geo: R. Hasenyager 833,980.21E SAMPLE TESTING TOPOGRAPHIC MAP INFORMATION: WATER LEVEL INFORMATION:												
2		L	1		_		TOPOGRAPHIC MAP INFORMATION: W Quadrangle: Joppa			ATER LEVEL INFORMATION: $\nabla = 43.45$ During Drilling			
ıber	Recov / Total (in) % Recovery	0	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Township: 15S;3E Section 14, Tier 15S; Range 3E			$\Psi = 43.45$ - During Drilling $\Psi = $ $\Psi = $ $\Psi = $			
Number	Recc % R	Type	Blow N - ' RQI	Moi	Dry	Qu (Failt	Depth ft. BGS	Lithologic Description	ł	Borehole Detail	Elevation ft. MSL	Remarks	
11A	24/24 100%	ss	1-3 4-5 N=7	20			¥ 44	Dark yellowish orange (10YR6/6) with 20% gray (10YR5/1) mottles, wet, dense, very fine- to medium-grained SAND with some silt, little clay, and to small gravel. [Continued from previous page]	ace		310		
12A	21/24 88%	ss	2-5 12-14 N=17	16			46	Yellowish brown (10YR5/8) with 10% gray (10YR6/	10		306		
13A	23/24 96%	ss	1-1 6-10 N=7	20			46	mottles, wet, dense, very fine- to fine-grained SAND w some silt and trace clay.					
14A	15/24 63%	ss	wor-wor woh-3	25				Gray (10YR6/1) wet, loose, very fine- to medium-grain	ned		304		
15A	19/24 79%	ss	<i>1-3</i> <i>8-14</i> N=11	23			50	SAND (micaceous) with little silt and trace clay.			302		
16A	20/24 83%	ss	<i>1-4</i> 5-14 N=9	23				Gray (10YR6/1) wet, loose, very fine- to medium-grain SAND (micaceous) with little silt and trace clay and sn gravel.			300		
17A	Ī		4.10	22			54				298		
17B	19/24 79%	ss	4-10 14-20 N=24	16			56	Gray (10YR6/1), wet, loose, very fine- to very coarse-grained SAND with few small to large gravel a little silt.			296		
18B N=19 21 SAND with little silt						Gray (10YR6/1) wet, loose, very fine- to fine-grained SAND with little silt and trace small gravel.							
					I	I	58 =	End of boring = 58.0 feet			<u> </u>		



	CLIENT Sit	F: Na e: Jo	BOR atural Res ppa Powe ppa, Illin	sour er Sta	ce Te	chnolo	r, Inc. CONTRA Rig mfg	CTOR: Bulldog Drilling, Inc. /model: CME-750 ATV Drill fethod: 4¼" HSA, split spoon samp		OREHOLE ID: Well ID:				
	Projec DATES	t: 15 5: St Fir		1/20 1/20	15	ł	C C	TAFF: Driller: J. Dittmaier Helper: M. Hill Eng/Geo: S. Keim		Completion:	353.71 ft. MSL 80.14 ft. BGS 199,066.83N 831,610.42E			
5	SAMPL	Ε	Т	EST	TING		TOPOGRAPHIC MAP INFO	DRMATION:	WATER LEVE					
	Recov / Total (in) % Recovery		in	(%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadrangle: Joppa Township: 15S;3E	vnship: 158;3E		Ψ = 18.00 - During Drilling Ψ = ∇ =				
Number	cov / J Recove	Type	Blows / 6 in N - Value RQD	Moisture (%)	y Den.	i (tsf) ilure T	Section 14, Tier 15S; Rang Depth	Lithologic	<u>⊥</u> = Borehol	e Elevation				
Nu	Re %	<u>T</u>	R N Bl	Ŭ	D	Qu Fa	ft. BGS FILL - Black (10	Description YR2/1), moist, medium, SILT with f	Detail	ft. MSL	Remarks			
1A	13/24 54%	ss	3-2 3-4 N=5	21			Clay, trace very FILL - Yellowish with few clay, tra	fine- to medium-grained sand, and tra roots. brown (10YR5/4), moist, medium, S ce very fine- to coarse-grained sand, a trace roots.		352				
2A	16/24 67%	ss	2-4 16-8 N=20	13			(10YR5/1), moist fine- to coarse-gra	rish brown (10YR5/4) with 25% gray , medium, SILT with few clay, trace v ined sand, trace small to large gravel, trace roots.	verv 🖾 🗄	350				
3A	18/24 75%	ss	4-4 5-8 N=9	23		2.00	Light gray (10Y	R7/1) with 20% dark yellowish brow	vn vn					
4A	21/24 88%	ss	2-2 4-4 N=6	25		2.00	6 - (10YR4/6) mottl ver	es, moist, stiff, SILT with few clay, tra y fine- to fine-grained sand.						
5A	24/24 100%	ss	<i>1-3</i> <i>4-6</i> N=7	23		1.80	8		دي دي دي دي دي دي دي د	344				
6A	22/24 92%	ss	<i>1-3</i> <i>4-5</i> N=7	22		3.00	10 (10YR4/6) mottl	es, moist, stiff, SILT with few clay, travery fine-grained sand.		- 342				
7A	17/24 71%	ss	<i>1-1</i> <i>2-4</i> N=3	22		2.00	12	5/1) with 35% dark yellowish brown		340				
8A	22/24 92%	ss	<i>1-2</i> <i>4-5</i> N=6	22		2.00	14 (10YR4/6) mottl	es, moist, stiff, SILT with few clay, travery fine-grained sand.		338				
9A	17/24 71%	ss	2-3 6-8 N=9	22		1.80				- 336				
10A	24/24 100%	ss	2-3 6-5 N=9	20		2.50	mottles, moist, stif	n (10YR5/6) with 30% gray (10YR6/ f, silty CLAY with trace very fine-gra sand.	(1) ined					
	24/24 DTE(S):	 G54	<i>1-3</i> D installe	20 ed in	bore	<i>1.90</i> hole.	20			- 334				
											Page 1 of 4			

1	CLIENT: Natural Resource Technology, Inc. CONTRACTOR: Bulldog Drilling, Inc. Site: Joppa Power Station Rig mfg/model: CME-750 ATV Drill Location: Joppa, Illinois Drilling Method: 4¼" HSA, split spoon sampler Project: 15E0030 FIELD STAFF: Driller: J. Dittmaier Finish: 8/11/2015 Helper: M. Hill /EATHER: Sunny, warm, Io-80s Eng/Geo: S. Keim										Weil ID: G54D Surface Elev: 353.71 ft. MSL Completion: 80.14 ft. BGS Station: 199,066.83N 831,610.42E		
	SAMPL		-		ING	ř	TOPOGR	APHIC MAP INFORMATION:	WATE	R LEVEL	INFORMAT	TION:	
er	Recov / Total (in) % Recovery		/ 6 in lue	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Towns	angle: Joppa hip: 158;3E h 14, Tier 158; Range 3E	Ā	$\mathbf{\Psi} = 18.00$ - During Drilling $\mathbf{\Psi} = \mathbf{\nabla} = \mathbf{\nabla}$			
Number	Recov % Rec	Type	<i>Blows / 6 i</i> N - Value RQD	Moist	Dry D	Qu (ts Failure	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks	
	100% 24/24	SS SS	5-6 N=8				22	Yellowish brown (10YR5/6) with 30% gray (10YR6/1 mottles, moist, stiff, silty CLAY with trace very fine-grai sand. [Continued from previous page]	ned		332		
12A	100%	ss ss	6-8 N=10	19		3.00	22 24 26 28 30			5555 5555	330		
13A	24/24 100%	ss	4-6 N=7	19		1.80	26-			نے کے لیے لیے ل	328		
14A	24/24 100%	ss	2-4 7-7 N=11	21		2.00	28	Light brownish gray (10YR6/2) with 35% dark yellowi brown (10YR4/6) mottles, moist, very stiff, SILT with f clay, trace very fine- to medium-grained sand.	sh èw	5555	326		
15A	22/24 92%	ss	1-3 6-8 N=9	20		2.30	30			ي تي تي تي تي برتي تي تي	324		
16A	20/24 83%	ss	1-3 6-6 N=9	18		2.40				د د د د د	- 322		
17A	24/24 100%	ss	<i>1-4</i> 8-8 N=12	20		3.50	32			د و و و و و د و و و و و و د ر و و و و	320		
18A	22/24 92%	ss	1-3 4-5 N=7	22		2.80	36	Grayish brown (10YR5/2) with 10% dark yellowish bro (10YR4/6) mottles, moist, very stiff, SILT with little cla trace very fine-grained sand.	wn ay,	نے کے لیے لیے لیے نے لیے لیے لیے لیے	318		
19A	24/24 100%	ss	3-4 8-8 N=12	19		3.00	38				316		
20A	24/24 100%	ss	1-4 7-10 N=11	19		2.50	36 36 38 40 42	Light gray (10YR7/1) with 5% yellowish brown (10YR5/6) mottles, moist, very stiff, SILT with few cla trace very fine-grained sand.	y,	ے فی فی فی فی و ی فی فی فی و	314		
21A	24/24 100%	ss	1-3 10-10 N=13	17		3.80				ت _ہ تہ تہ تہ ت	312		
NC	NOTE(S): G54D installed in borehole.												

Page 2 of 4

	CLIENT Sit Location Projec DATES	f: Na e: Jo n: Jo t: 15 5: St Fin	BOR atural Re pppa Powe pppa, Illin 5E0030 tart: 8/1 hish: 8/1 unny, war	soure er Sta ois 1/20 1/20	r BOREHOLE ID: G54D Surface Elev: 353.71 ft. MSL Completion: 80.14 ft. BGS Station: 199,066.83N 831,610.42E				
5	SAMPL	E	Т	EST	TING			RAPHIC MAP INFORMATION:	WATER LEVEL INFORMATION:
er	Recov / Total (in) % Recovery		/ 6 in lue	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Towns	rangle: Joppa ship: 15S;3E n 14, Tier 15S; Range 3E	$\mathbf{\Psi} = 18.00$ - During Drilling $\mathbf{\Psi} = \mathbf{\nabla} = \mathbf{\nabla}$
Number	Recov % Rec	Type	Blows / 6 in N - Value RQD	Moistı	Dry D	Qu (ts) Failure	Depth ft. BGS	Lithologic Description	Borehole Elevation Detail ft. MSL Remarks
22A	24/24 100%	ss	5-8 9-10 N=17	16		3.00	44 44 44 44 44 44 44 44 44 44 44 44 44	Light gray (10YR7/1) with 5% yellowish brown (10YR5/6) mottles, moist, very stiff, SILT with few clay	y, 310
23A	24/24 100%	ss	3-6 8-8 N=14	19		3.00	46	trace very fine-grained sand. [Continued from previous page]	
24A	24/24 100%	ss	2-4 5-5 N=9	19		3.20	48	Light brownish gray (10YR6/2) with 10% dark yellowis	sh 306
25A	24/24 100%	ss	4-7 7-6 N=14	18		2.40	50	brown (10YR474) mottles, moist, very stiff, silty CLAY trace very fine- to coarse-grained sand.	304
26A	17/24 71%	ss	<i>1-4</i> 6-8 N=10	18		2.00	52	Gray (10YR6/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, very stiff, silty CLAY with litt very fine- to coarse-grained sand and trace small gravel	
27A	22/24 92%	ss	3-3 7-18 N=10	21		3.30		Gray (10YR6/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, very stiff, SILT with few clay	v
28A	21/24 88%	ss	5-11 18-20 N=29	21		4.00		trace very fine-grained sand.	
29A	11/11 100%	ss	25-50/5	15		3.80	56	Gray (10YR6/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, hard, SILT with few very fine- fine-grained sand, trace clay.	- to
30A	20/24 83%	ss	21-19 35-29 N=54	14			58	Gary (10YR6/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, very dense, silty, very fine- to fine-grained sand, trace clay.	o 294
31A	17/24 71%	ss	4-6 10-9 N=16	17		1.80	54	Gray (10YR6/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, stiff, SILT with little clay, trac very fine- to fine-grained sand.	ce
32A NC		K G54	6-8 D installe	22 22	bore	<i>3.30</i> hole.			
									Page 3 of 4

	CLIENT Sit	F: Na e: Jo	BOR atural Re oppa Powe oppa, Illin	soure er Sta	ce Te	echnolo		CONTRACTOR: Bulldog Drilling, Inc. Rig mfg/model: CME-750 ATV Drill Drilling Method: 4¼" HSA, split spoon samp	ler	BO	REHOLE ID Well ID	
		S: St Fir	art: 8/1 hish: 8/1	1/20	15			FIELD STAFF: Driller: J. Dittmaier Helper: M. Hill			Completion	: 353.71 ft. MSL : 80.14 ft. BGS : 199,066.83N
	SAMPL		inny, wai T		5-80s TINC			Eng/Geo: S. Keim				831,610.42E
	Recov / Total (in) % Recovery		Blows / 6 in N - Value RQD		Dry Den. (lb/ft ³)	Qu (tsf) <i>Qp</i> (tsf) Failure Type	Quadr Towns	RAPHIC MAP INFORMATION: rangle: Joppa chip: 15S;3E n 14, Tier 15S; Range 3E		= 18.00 -	INFORMA During Drillin	
Number	Reco % Re	Type	Blow. N - V RQD	Mois	Dry I	Qu (t Failu	Depth ft. BGS	Lithologic Description		Borehole Detail	Elevation ft. MSL	Remarks
33A	92% 24/24 100%	ss	5-7 N=13 2-4 7-9 N=11	18		2.50	64	Gray (10YR6/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, stiff, SILT with little clay, tr very fine- to fine-grained sand. [Continued from previous page]	ace	د و و و و و و و و و و و د و و و و و و و	- 290	
34A	24/24 100%	ss	3-5 10-15 N=15	19		2.50		Gray (10YR6/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, stiff, SILT with little clay, t very fine- to coarse-grained sand, trace small gravel	èw		- 286	
35A	22/24 92%	ss	<i>1-3</i> <i>3-5</i> N=6	23		1.50	70	Yellowish brown (10YR5/6) with 10% gray (10YR6, mottles, moist, stiff, SILT with little clay, trace very fine-grained sand.	/1)		284	
36A	24/24 100%	ss	woh-1 5-6 N=6	23		1.50	72	Gray (10YR6/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, stiff, SILT with little clay, to very fine- to fine-grained sand.	ace			
37A	24/24 100%	ss	woh-1 2-4 N=3	23		1.50	74				- 280	
38A	16/24 67%	ss	30-40 27-34 N=67	11			76		0		278	
39A	12/15 80%	ss	10-47 50/3"	14			76	Yellowish brown (10YR5/6), moist, very dense, silty, f to coarse-grained SAND and small to large GRAVE	ine-		 	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$							0		- 274			
						ļ	80 –	End of boring = 80.14 feet	• • 5		÷- 2/7	
			D installe									

Illinois Environ	mental Protection Agen	cy		Wel	l Completion	Report
Site #:	County:	Massac Cou	nty	V	Well #: G0	1D
Site Name:Joppa Power Stati	on			E	Borehole #:	601D
State Plane Coordinate: X 831,716	5.1 Y <u>202,039.3</u> (or) Lati					
Surveyed By: <u>Gary C. Rogers</u>		IL Reg	istration #: <u>035-0</u>	02957		
Drilling Contractor: <u>Bulldog D</u>	rilling, Inc.	Driller	J. Dittmaier			
Consulting Firm: <u>Hanson Profe</u>	essional Services Inc.	Geolog	sist: <u>Rhonald W.</u>	Hasenyage	r, LPG #196-0002	246
Drilling Method:Hollow Stem	Auger	Drillin	g Fluid (Type): <u>W</u>	ater		
Logged By: <u>Suzanna L. Keim</u>		Date S	tarted: <u>8/14/20</u>)15 Dat	te Finished: <u>8/1</u>	4/2015
Report Form Completed By:Su	zanna L. Keim	Date:	8/18/2015			
ANNULAR SPA	CE DETAILS		Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
	-		364.50	-3.00	Top of Protective	Casing
	L		364.19	-2.69	Top of Riser Pipe	
Type of Surface Seal: <u>Concrete</u>			361.50	0.00	Ground Surface	
Type of Annular Sealant: <u>High-s</u>	solids bentonite		359.50	2.00	Top of Annular S	ealant
Installation Method:						
Setting Time:		\ ⊥	_311.42_	50.08	Static Water Leve (After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry — (choose one)					
Installation Method: <u>Gravit</u>	у		310.15	51.35	Top of Seal	
Setting Time: <u>>48 hours</u>			308.90	52.60	Top of Sand Pack	
Type of Sand Pack: <u>Quartz San</u> Grain Size: <u>10-20</u> (sie			307.31	54.19	Top of Screen	
Installation Method: <u>Gravit</u> Type of Backfill Material: <u>n/a</u>			<u>297.65</u> 297.12	<u>63.85</u> <u>64.38</u>		
Installation Method:			297.12 * Referenced to a	64.38 A National Geode		le
			C 44		CI ID EM ENTO	
			Diameter of Boreh		SUREMENTS (inches)	8.0
	STRUCTION MATERIALS e type of material for each area)		ID of Riser Pipe		, , ,	2.0
			Protective Casing I			5.0
Protective Casing	SS304 SS316 PTFE PVC OT	HER: (Steel)	Riser Pipe Length		(feet)	<u>56.88</u> 0.53
Riser Pipe Above W.T.		HER:	Bottom of Screen t Screen Length (1			9.66
Riser Pipe Below W.T.	SS304 SS316 PTFE PVC OT	HER:	Total Length of Ca		· · · ·	67.07
Screen	SS304 SS316 PTFE PVC OT	HER:	Screen Slot Size **		(inches)	0.010

Illinois Environ	mental Protection Age	ncy			Wel	l Completion	Report
Site #:	County:	Massao	c Coun	ty	V	Vell #: G0	2D
Site Name: Joppa Power Stati						orehole #: (602D
State	<u>3.0 Y 202,137.1 (or) L</u>						
Surveyed By: <u>Gary C. Rogers</u>				tration #:035-0			
	rilling, Inc.		Driller:	J. Dittmaier			
	essional Services Inc.					r, LPG #196-0002	
Drilling Method: Hollow Stem			-				
Logged By: <u>Suzanna L. Keim</u>	-		-			e Finished:8/1	
							5/2015
Report Form Completed By: <u>Su</u>		1	Date	8/18/2015			
ANNULAR SPA	CE DETAILS			Elevations (MSL)*	Depths (BGS)	(0.01 ft.)	
				364.09	3.27	Top of Protective	Casing
				363.65	-2.83	Top of Riser Pipe	
Type of Surface Seal: <u>Concrete</u>					0.00	Ground Surface	
				358.82	2.00	Top of Annular S	ealant
Type of Annular Sealant: <u>High-</u>	solids bentonite	M .	TY .			T	
Installation Method:	2						
Setting Time: <u>>48 hours</u>				312.82	48.00	Static Water Leve (After Completion)	
Type of Bentonite Seal Gran	ular Pellet Slurry (choose one)	\pm					
Installation Method:	. ,			301.82	59.00	Top of Seal	
Setting Time: <u>45 minutes</u>				300.42	60.40	Top of Sand Pack	
Type of Sand Pack: <u>Quartz San</u>				298.61	62.21	Top of Screen	
Grain Size: 10-20 (sie						Top of Screen	
Installation Method: <u>Gravit</u>	У			288.98	71.84	Bottom of Screen	
Type of Backfill Material: <u>n/a</u>	(if applicable)			288.46	72.36		
Installation Method:				288.46	72.36	Bottom of Boreho	ble
				* Referenced to a			
				CAS	SING MEA	SUREMENTS	
				Diameter of Boreho		(inches)	8.0
	e type of material for each area)			ID of Riser Pipe		(inches)	2.0
			Ļ	Protective Casing L	ength	(feet)	5.0
				Riser Pipe Length		(feet)	65.04
Protective Casing		OTHER: Stee	—	Bottom of Screen to	-		0.52
Riser Pipe Above W.T. Riser Pipe Below W.T.	SS304SS316PTFEPVCSS304SS316PTFEPVC	OTHER:		Screen Length (19		, , , , , , , , , , , , , , , , , , ,	9.63
Screen	SS304 SS316 PTFE (PVC)			Total Length of Ca	-		75.19
				Screen Slot Size **		(inches)	0.010

Illinois Environmental Protection Agency		Well Completion Report
Site #: County:	ssac County	Well #: G51D
Site Name: Joppa Power Station		Borehole #:G51D
State Plane Coordinate: X 832,151.5 Y 200,430.1 (or) Latitude:	La	ongitude:
Surveyed By: <u>Gary C. Rogers</u>	IL Registration #:035-00295	57
Drilling Contractor: Bulldog Drilling, Inc.	Driller: J. Dittmaier	
Consulting Firm: <u>Hanson Professional Services Inc.</u>	Geologist: <u>Rhonald W. Hase</u>	enyager, LPG #196-000246
Drilling Method: Hollow Stem Auger	Drilling Fluid (Type): <u>Water</u>	
Logged By: <u>Rhonald W. Hasenyager</u>	Date Started: <u>8/17/2015</u>	Date Finished:8/18/2015
Report Form Completed By: <u>Suzanna L. Keim</u>	Date: <u>8/28/2015</u>	-
ANNULAR SPACE DETAILS		epths (0.01 ft.) 3GS)
		3.12 Top of Protective Casing
		2.75 Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	361.10 0	0.00 Ground Surface
Ture of Annular Scalant. High solids bontonits		2.00 Top of Annular Sealant
Type of Annular Sealant: <u>High-solids bentonite</u> Installation Method: <u>Tremie</u>		
	⊴	9.45 Static Water Level (After Completion) 10/5/2015
Type of Bentonite Seal Granular Pellet Slurry (choose one) Installation Method: Gravity	4	5.68 Top of Seal
Setting Time: 70 minutes		
	<u>× 313.59 4</u>	7.51 Top of Sand Pack
Type of Sand Pack: <u>Quartz Sand</u> Grain Size: <u>10-20</u> (sieve size)	4	9.61 Top of Screen
Installation Method: <u>Gravity</u> Type of Backfill Material: <u>n/a</u> (if applicable)		9.27Bottom of Screen9.90Bottom of Well
(if applicable) Installation Method:	301.2059	9.90 Bottom of Borehole
	CASING	MEASUREMENTS
	Diameter of Borehole	(inches) 8.0
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)	ID of Riser Pipe	(inches) 2.0
	Protective Casing Length	
Protective Casing SS304 SS316 PTFE PVC OTHER: (Riser Pipe Length	(feet) 52.36
Protective Casing SS304 SS316 PTFE PVC OTHER: (Riser Pipe Above W.T. SS304 SS316 PTFE PVC OTHER: (
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER:	Screen Length (1st slot f Total Length of Casing	
Screen SS304 SS316 PTFE PVC OTHER:	Screen Slot Size **	(inches) 0.010

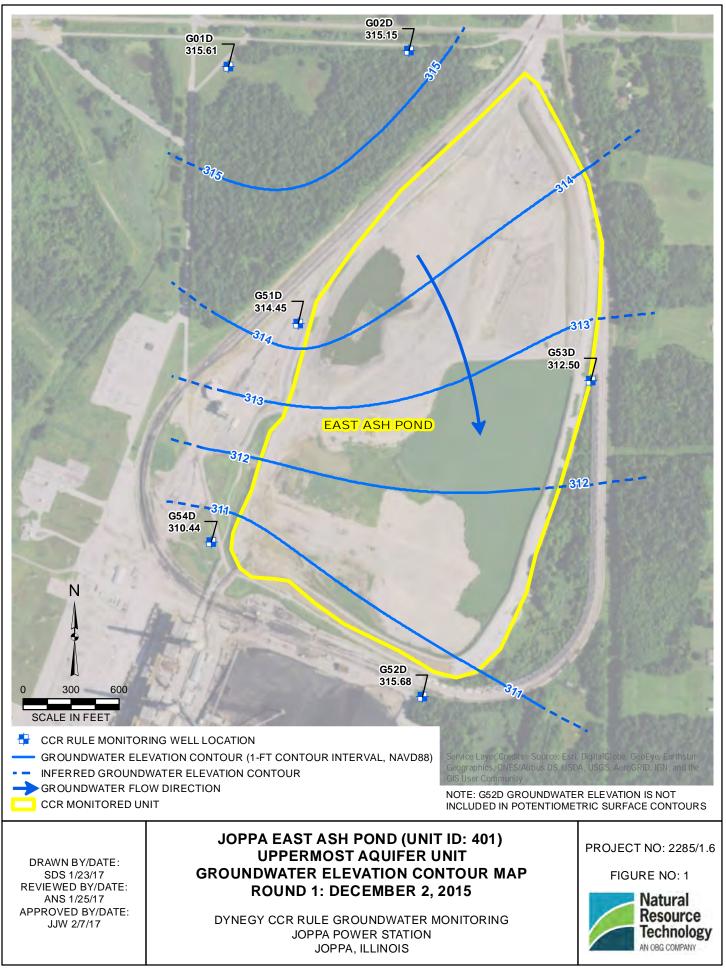
Illinois Environmental Protection A	gency			Well	Completion	Report
Site #: Cou	nty: <u>Massac</u>	c Count	y	W	Vell #:65	2D
Site Name: Joppa Power Station				В	orehole #:	G52D
State Plane Coordinate: X 832,927.9 Y 198,098.9 (or)	Latitude:			Longitud	e:	
Surveyed By: <u>Gary C. Rogers</u>			ration #: <u>035-0</u>			
Drilling Contractor:Bulldog Drilling, Inc.	I	Driller:	J. Dittmaier			
Consulting Firm: <u>Hanson Professional Services Inc.</u>	(Geologist	: <u>Rhonald W.</u>	Hasenyage	r, LPG #196-000	246
Drilling Method: <u>Hollow Stem Auger</u>	I	Drilling F	luid (Type): <u>Wa</u>	ater		
Logged By: <u>Rhonald W. Hasenyager</u>	I	Date Star	ted: <u>8/18/20</u>	<u>15</u> Dat	e Finished: <u>8/</u>	19/2015
Report Form Completed By: <u>Suzanna L. Keim</u>	I	Date:	8/28/2015			
ANNULAR SPACE DETAILS			Elevations	Depths	(0.01 ft.)	
			(MSL)* 348.67	(BGS) -2.79	Top of Protective	Casing
			348.41	-2.53	1	
Type of Surface Seal: <u>Concrete</u>	-		345.88	0.00	Ground Surface	
Type of Annular Sealant: High-solids bentonite		H	343.88	2.00	Top of Annular S	ealant
Installation Method:						
Setting Time:	Į		313.46	32.42	Static Water Leve (After Completion)	
Turne of Dentemits Seel Conversion (Dellet) Slower					(Anter Completion)	10/5/2015
Type of Bentonite Seal Granular Pellet Slurry (choose one)		\prod				
Installation Method: <u>Gravity</u>	\mathbf{X}	\mathbf{X}	278.91	66.97	Top of Seal	
Setting Time: <u>32 minutes</u>	×	×	277.22	68.66	Top of Sand Pack	X
Type of Sand Pack:Quartz Sand						
Grain Size: <u>10-20</u> (sieve size)			276.03	69.85	Top of Screen	
Installation Method:Gravity						
Type of Backfill Material: <u>n/a</u>			<u>266.33</u> 265.87	<u>79.55</u> 80.01	Bottom of Screen Bottom of Well	
(if applicable)		_				
Installation Method:			265.87 * Referenced to a	80.01 National Geodet	Bottom of Boreho	ble
			CAS	ING MEA	SUREMENTS	
		Γ	Diameter of Boreho		(inches)	8.0
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)		Ι	D of Riser Pipe		(inches)	2.0
		F	Protective Casing L	ength	(feet)	5.0
Destastiva Casing S204 S216 DTEC DVG			Riser Pipe Length		(feet)	72.38
Protective Casing SS304 SS316 PTFE PVC Riser Pipe Above W.T. SS304 SS316 PTFE PVC			Bottom of Screen to		(feet)	0.46
Riser Pipe Below W.T. SS304 SS316 PTFE PVC			Coreen Length (1s		· · · ·	<u>9.70</u> 82.54
	OTHER:		Screen Slot Size **	-	(feet) (inches)	0.010

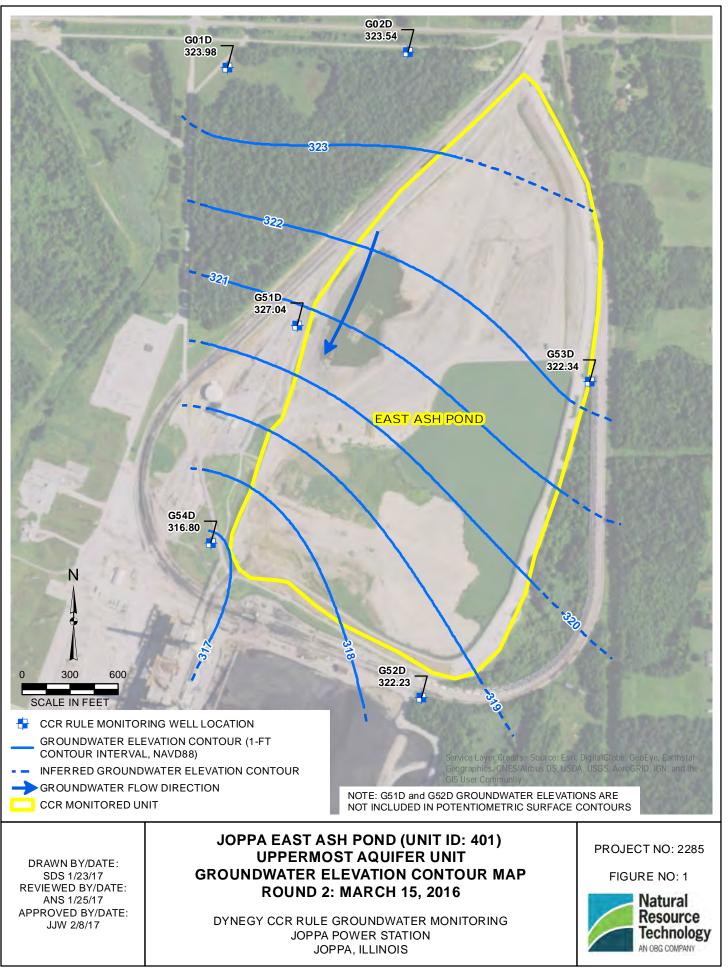
Illinois Environmental Protection Agency	Well Completion Report				
Site #: County:	ssac County	Well #: G53D			
Site Name: Joppa Power Station					
State Plane Coordinate: X 833,980.2 Y 200,075.2 (or) Latitude:					
Surveyed By: <u>Gary C. Rogers</u>		2957			
Drilling Contractor: Bulldog Drilling, Inc.	Driller: J. Dittmaier				
Consulting Firm: <u>Hanson Professional Services Inc.</u>		lasenyager, LPG #196-000246			
	_				
Drilling Method: Hollow Stem Auger		er			
Logged By: <u>Rhonald W. Hasenyager</u>		5 Date Finished: <u>8/21/201</u>	5		
Report Form Completed By: <u>Suzanna L. Keim</u>	Date: <u>8/28/2015</u>				
ANNULAR SPACE DETAILS	Elevations (MSL)*	Depths (0.01 ft.) (BGS)			
		<u>-3.66</u> Top of Protective Casing	ç.		
		-3.31 Top of Riser Pipe			
Type of Surface Seal: <u>Concrete</u>	352.16	0.00 Ground Surface			
	_350.16	2.00 Top of Annular Sealant			
Type of Annular Sealant: <u>High-solids bentonite</u>					
Installation Method: <u>Tremie</u>		40.05 and Mar 1			
Setting Time: <u>>48 hours</u>	⊻	42.25 Static Water Level (After Completion) 10/6/201:	5		
Type of Bentonite Seal Granular Pellet Slurry (choose one)					
Installation Method: <u>Gravity</u>		42.39 Top of Seal			
Setting Time: <u>65 minutes</u>		44.31 Top of Sand Pack			
Type of Sand Pack:Quartz Sand					
Grain Size: <u>10-20</u> (sieve size)		47.29 Top of Screen			
Installation Method: <u>Gravity</u>	205.27	56.00 D. H. 100			
Type of Backfill Material: Formation	<u>295.27</u> <u>294.83</u>	56.89Bottom of Screen57.33Bottom of Well			
(if applicable)	204.16	59.00 Dettem of Derekale			
Installation Method: Drilling	294.16 * Referenced to a N	58.00 Bottom of Borehole			
	CASI	NG MEASUREMENTS			
	Diameter of Borehole)		
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)	ID of Riser Pipe	(inches) 2.0			
	Protective Casing Let				
Protective Casing SS304 SS316 PTFE PVC OTHER: (Riser Pipe Length	(feet) 50.6			
Protective Casing SS04 SS10 PTPE PVC OTHER: Riser Pipe Above W.T. SS304 SS316 PTFE PVC OTHER:					
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER:	Screen Length (1st state) Total Length of Casin Total Length of Casin	· · · · ·			
Screen SS304 SS316 PTFE PVC OTHER:	Screen Slot Size **		010		

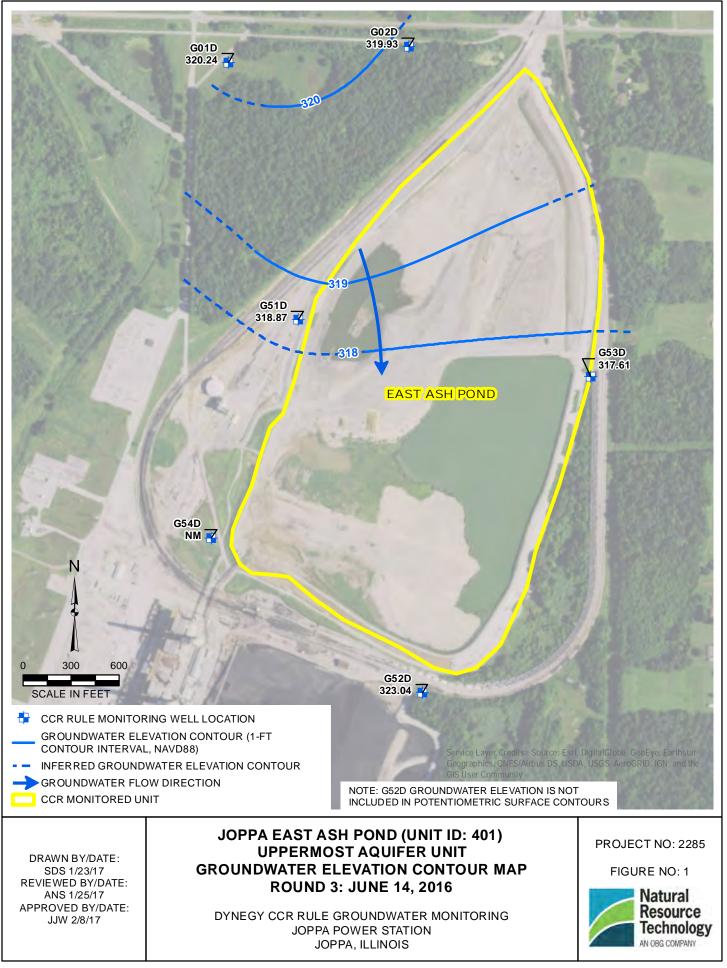
^{**}Hand-Slotted Well Screens Are Unacceptable

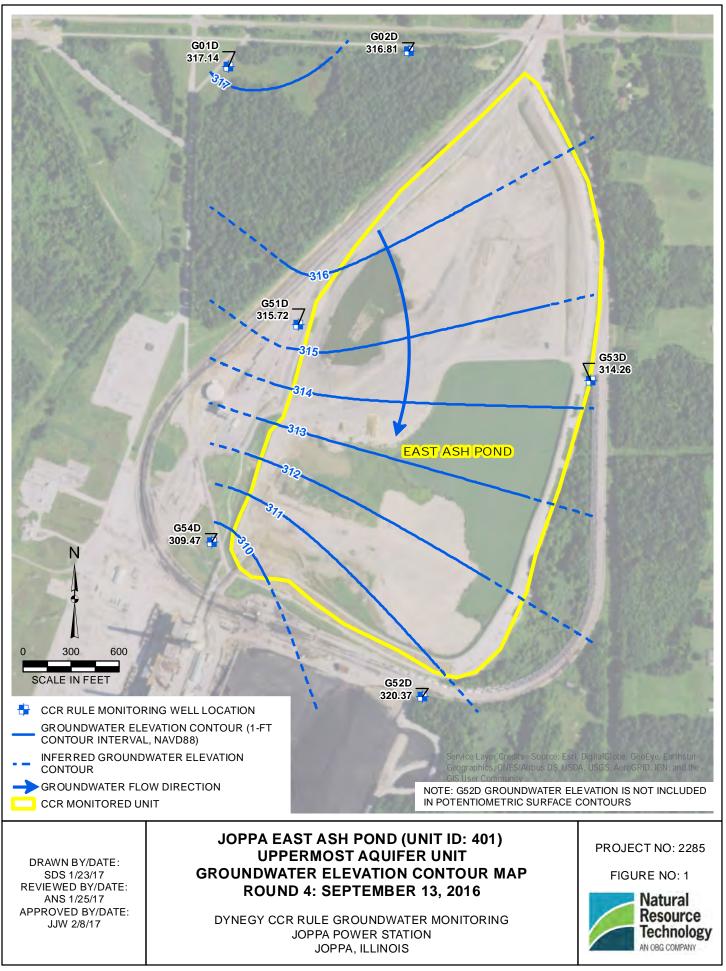
Illinois Environmental Protection Agency		Well Completion Report
Site #: County: Mas	sac County	Well #: G54D
Site Name: Joppa Power Station		Borehole #:G54D
State Plane Coordinate: X 831,610.4 Y 199,066.8 (or) Latitude:		
Surveyed By: <u>Gary C. Rogers</u> Gary C. Rogers	IL Registration #:	
Drilling Contractor: Bulldog Drilling, Inc.	Driller: J. Dittmaier	
Consulting Firm: <u>Hanson Professional Services Inc.</u>	Geologist: <u>Rhonald W. F</u>	lasenyager, LPG #196-000246
Drilling Method: Hollow Stem Auger	Drilling Fluid (Type): <u>Wat</u>	ter
Logged By: <u>Suzanna L. Keim</u>	Date Started: <u>8/11/201</u>	5 Date Finished: 8/11/2015
Report Form Completed By: <u>Suzanna L. Keim</u>	Date: <u>8/18/2015</u>	
ANNULAR SPACE DETAILS	Elevations (MSL)*	Depths (0.01 ft.) (BGS)
		-3.68 Top of Protective Casing
		-3.32 Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	353.71	0.00 Ground Surface
	351.71	2.00 Top of Annular Sealant
Type of Annular Sealant: <u>High-solids bentonite</u>		••• •• •• •••
Installation Method:		
Setting Time: <u>>48 hours</u>		49.21 Static Water Level (After Completion) 10/5/2015
Type of Bentonite Seal Granular Pellet Slurry		
Installation Method: <u>Gravity</u>		66.95 Top of Seal
Setting Time:		68.00 Top of Sand Pack
Type of Sand Pack:Quartz Sand	283.75	69.96 Top of Screen
Grain Size: <u>10-20</u> (sieve size)		
Installation Method: <u>Gravity</u>	274.05	79.66 Bottom of Screen
Type of Backfill Material:	273.57	80.14 Bottom of Well
(IT applicable)	273.57	80.14 Bottom of Borehole
		lational Geodetic Datum
	CASI	NG MEASUREMENTS
	Diameter of Borehole	
WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)	ID of Riser Pipe	(inches) 2.0
		ngth (feet) 5.0
Protective Casing SS304 SS316 PTFE PVC OTHER:	Riser Pipe Length	(feet) 73.28 End Cap (feet) 0.48
Riser Pipe Above W.T. SS304 SS316 PTFE PVC OTHER:		End Cap(feet)0.48slot to last slot)(feet)9.70
Riser Pipe Below W.T. SS304 SS316 PTFE PVC OTHER:	Total Length of Casi	
Screen SS304 SS316 PTFE PVC OTHER:	Screen Slot Size **	(inches) 0.010

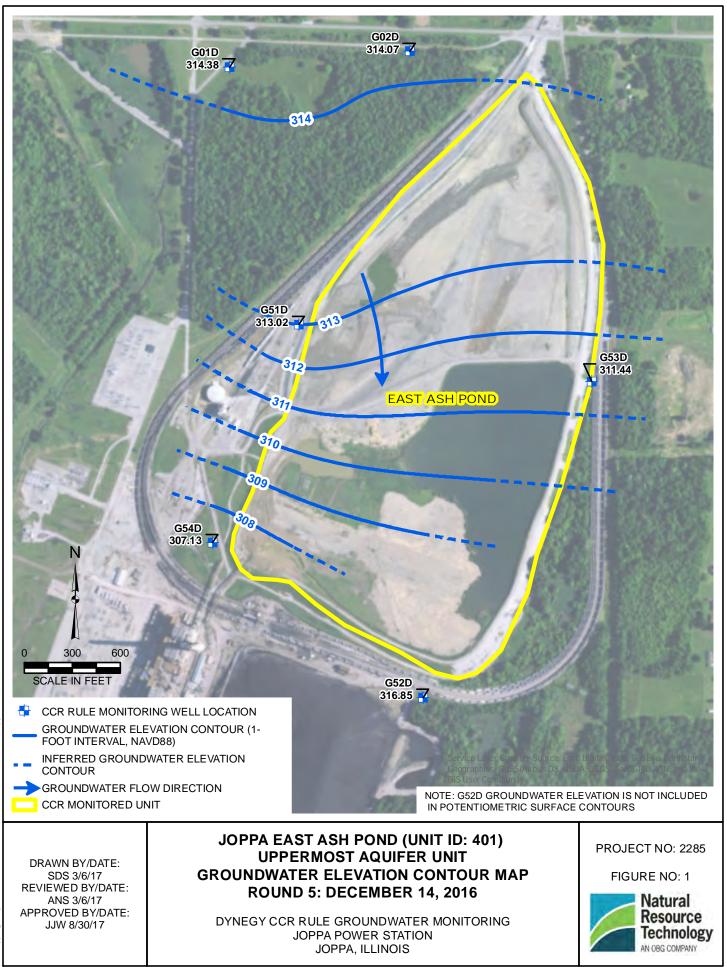
ATTACHMENT 4 – MAPS OF THE DIRECTION OF GROUNDWATER FLOW

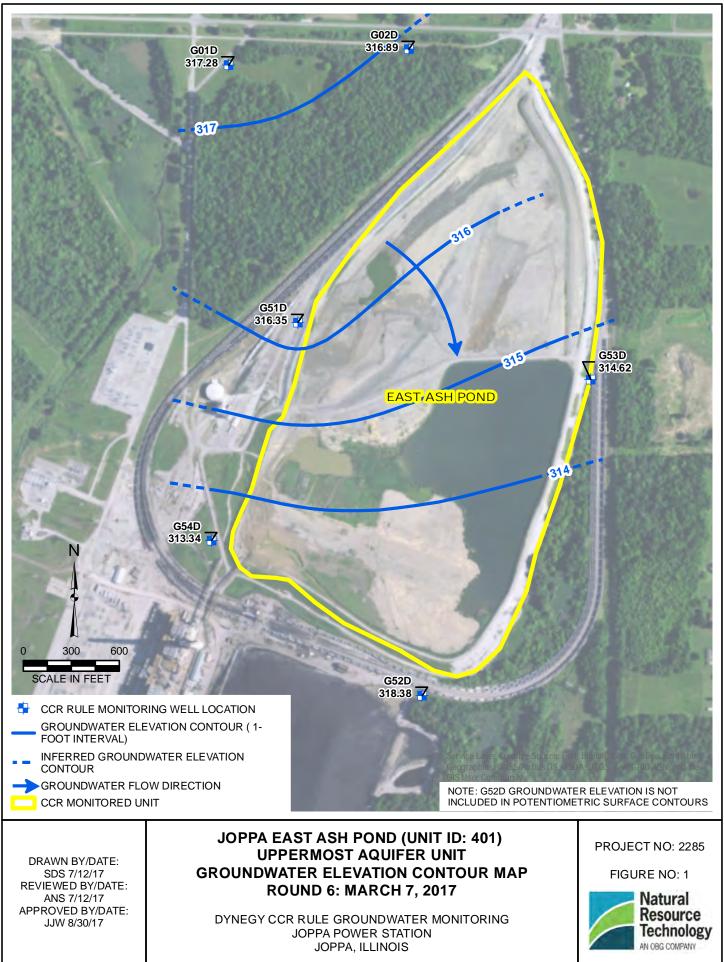


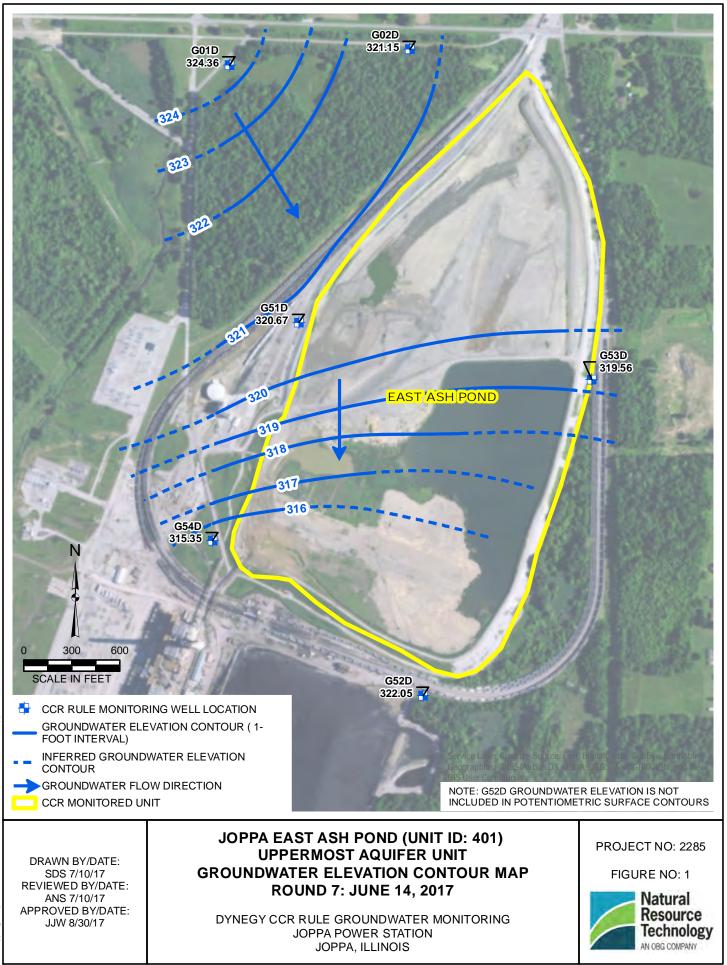


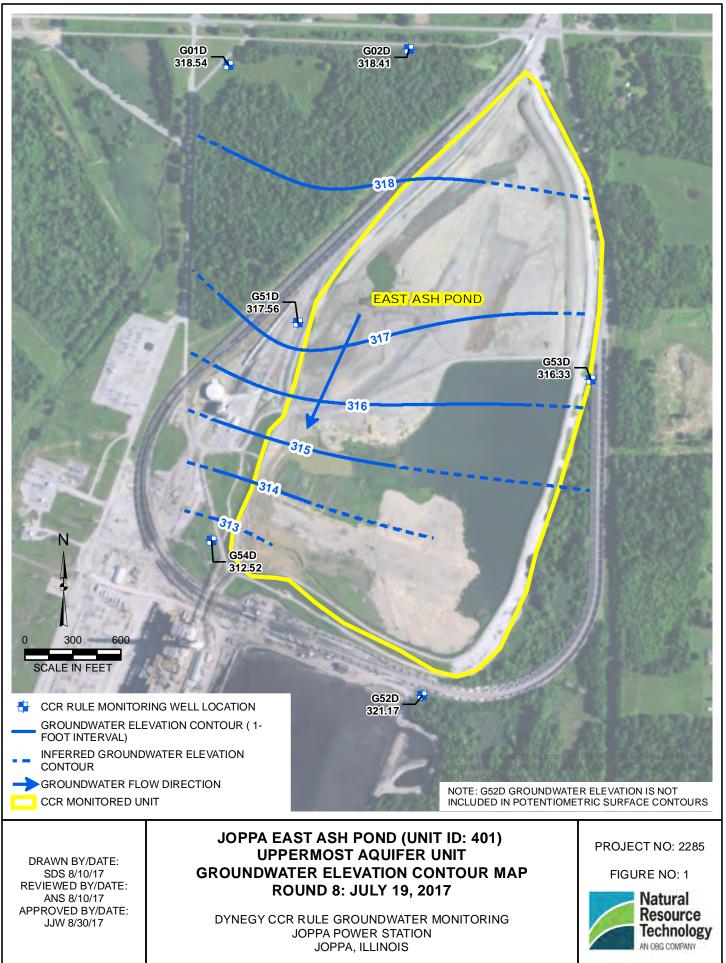


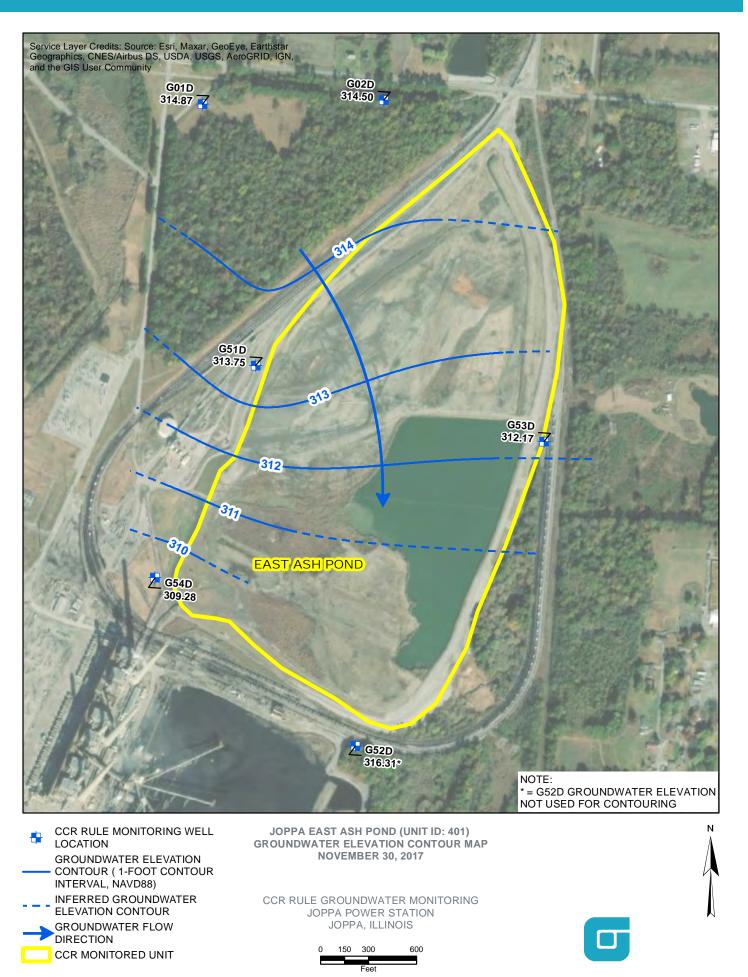


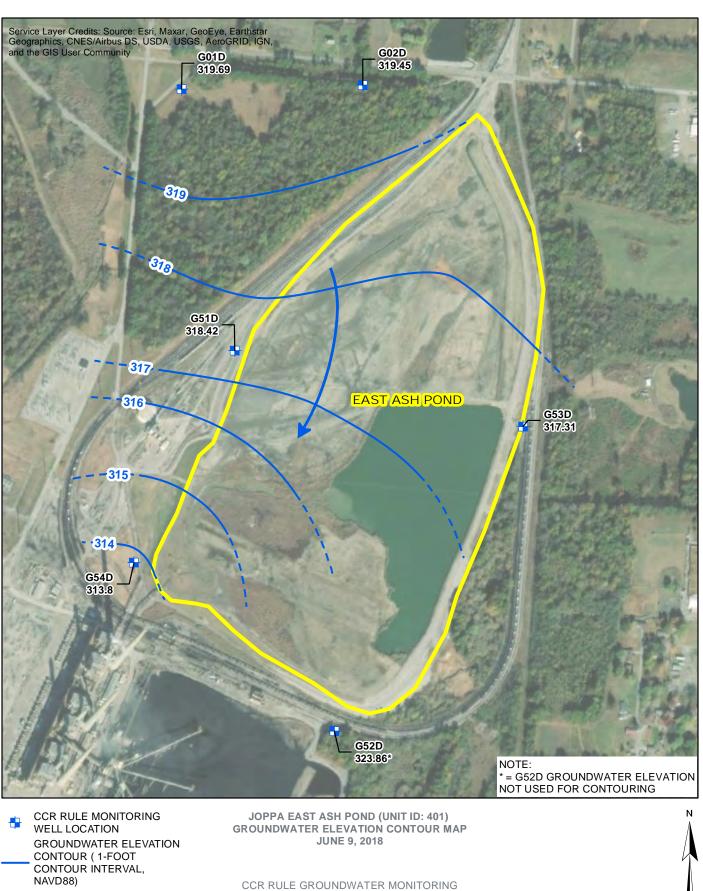










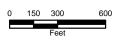


NAVD88)

INFERRED GROUNDWATER
ELEVATION CONTOUR

GROUNDWATER FLOW
DIRECTION

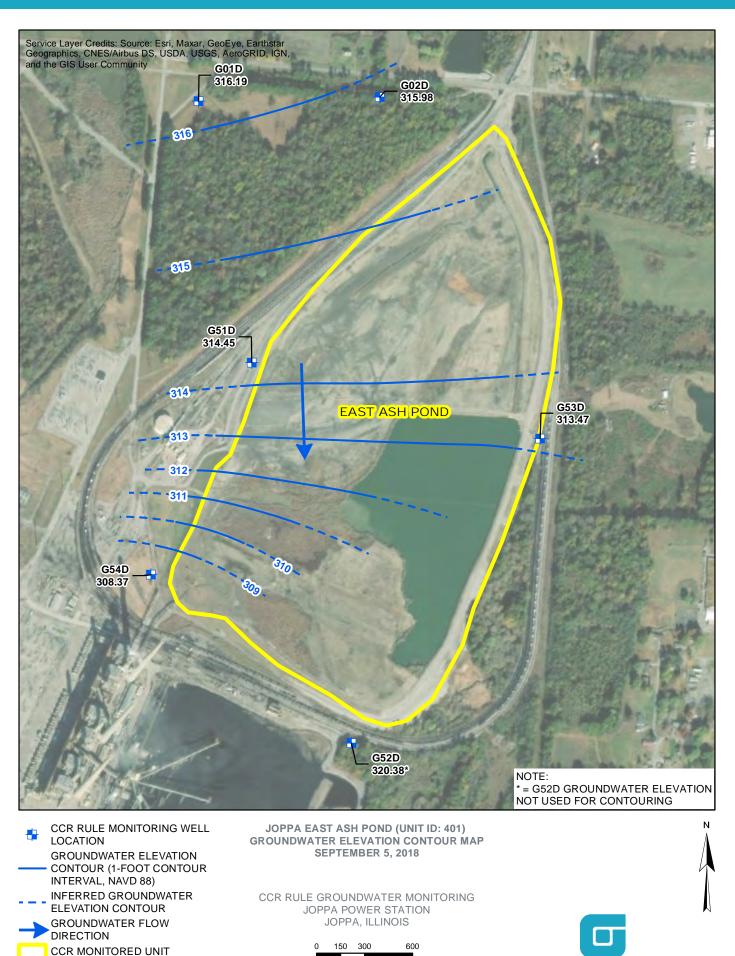
CCR MONITORED UNIT



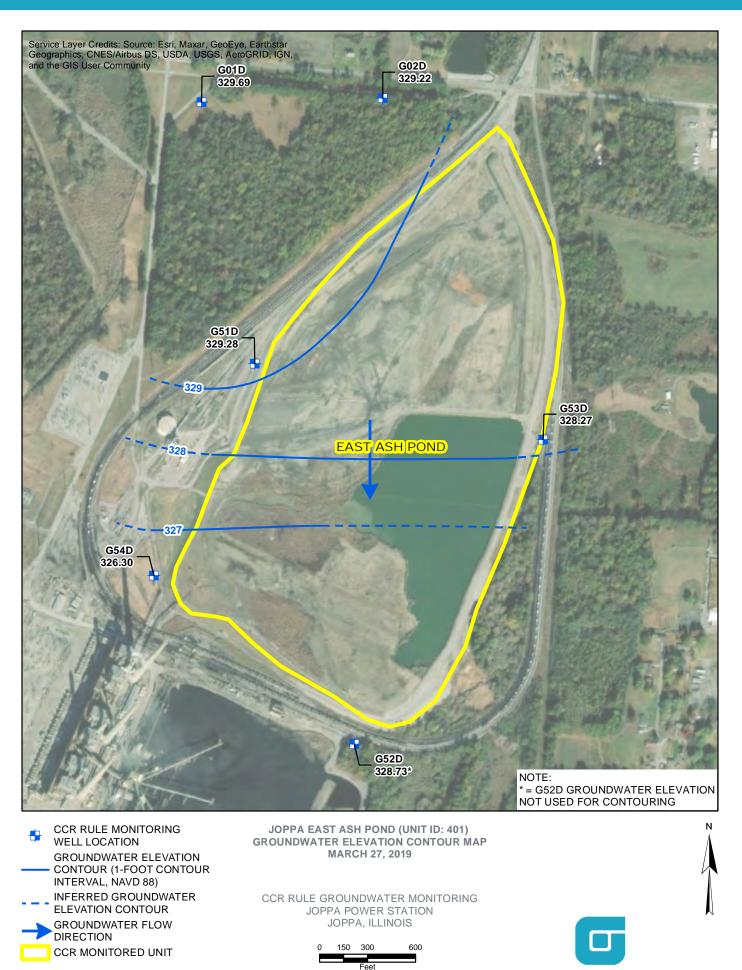
JOPPA POWER STATION

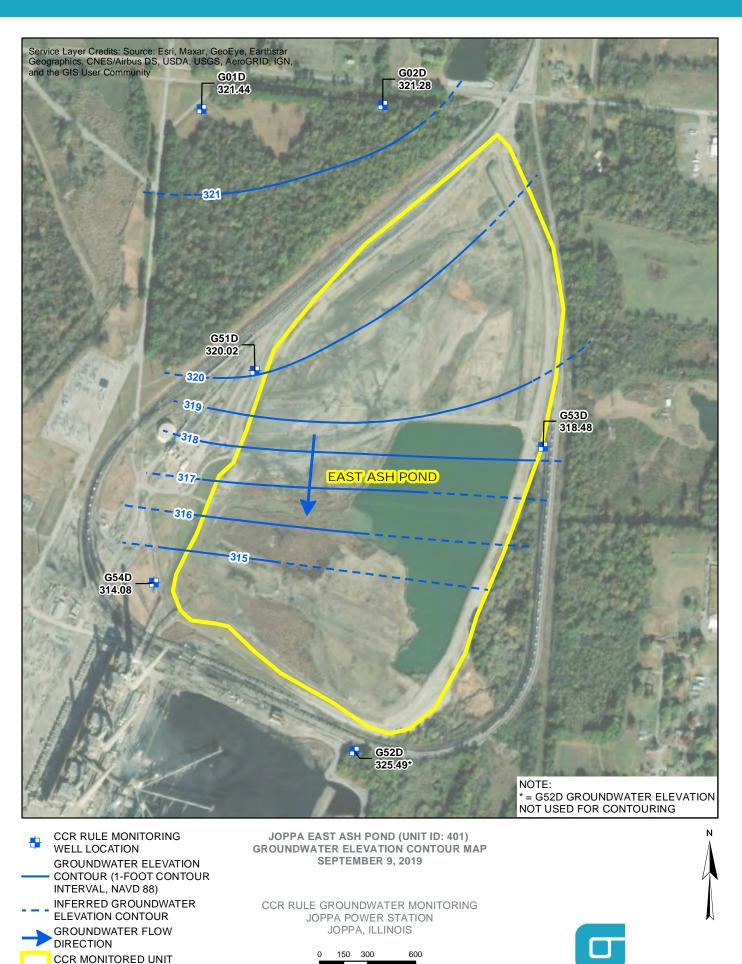
JOPPA, ILLINOIS



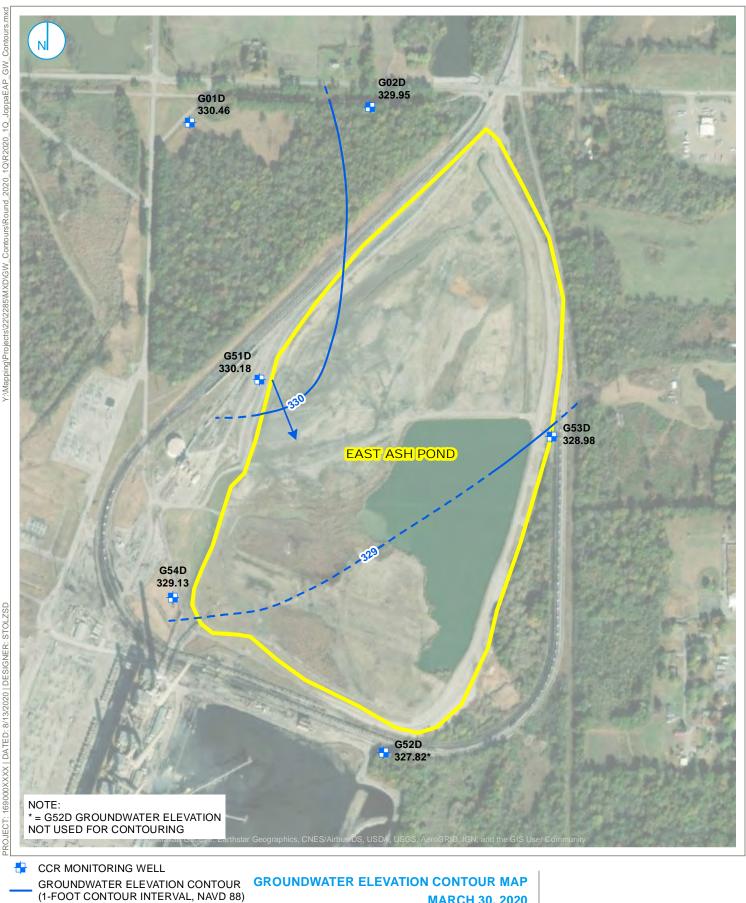


Feet





Feet



INFERRED GROUNDWATER ELEVATION CONTOUR GROUNDWATER FLOW DIRECTION

CCR UNIT BOUNDARY, SUBJECT SITE

MARCH 30, 2020

JOPPA EAST ASH POND (UNIT ID: 401) VIŠTRA ENERGÝ JOPPA POWER STATION JOPPA, ILLINOIS RAMBOLL US CORPORATION A RAMBOLL COMPANY



275 550 J Feet н

ATTACHMENT 5 – TABLES SUMMARIZING CONSTITUENT CONCENTRATIONS AT EACH MONITORING WELL

Analytical Results - Appendix III Joppa East Ash Pond

			Calcium,	Chloride,	Fluoride,		Sulfate,	Total	
Commis	Dete	Boron, total	total	total	total	рН	total	Dissolved	
Sample Location	Date Sampled	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(s.u.)	(mg/L)	Solids (mg/L)	
Background		(ing/L)	(iiig/L)	(iiig/L)	(ing/L)	(5.u.)	(ing/L)	(iiig/L)	
G01D	12/3/2015	<0.025	37.9	13	0.26	6.7	20	216	
G01D G01D	3/15/2016	0.025	45.5	20	0.20	6.7	126	496	
G01D G01D	6/15/2016	0.0296	43.9	20	0.29	6.9	120	518	
G01D G01D	9/14/2016	0.0230	40.8	21	0.25	6.8	129	498	
G01D G01D	12/14/2016	<0.025	35.9	14	0.20	6.8	53	294	
G01D G01D	3/7/2017	<0.025	34.9	16	0.24	6.2	72	384	
G01D G01D	6/15/2017	<0.025	32.1	15	0.22	6.7	56	372	
G01D G01D	7/20/2017	<0.025	29.5	12	0.24	6.8	31	368	
G01D G01D	11/30/2017	<0.025	37.2	12	0.24	6.8	117	450	
G01D G01D	6/19/2018	<0.025	29.5	13	0.22	6.8	70	394	
G01D G01D	9/5/2018	<0.025	30.5	14	0.24	7.0	94	414	
G01D	3/27/2019	<0.025	25.1	8	0.23	6.7	30	310	
G01D G01D	9/9/2019	<0.025	25.6	8	0.23	6.4	37	336	
G01D G01D	3/30/2020	<0.025	23.0	8	0.23	6.8	35	296	
G02D	12/3/2015	0.0536	39.9	24	0.24	6.7	16	244	
G02D G02D	3/15/2016	0.0494	39.8	24	0.24	6.6	17	256	
G02D G02D	6/15/2016	0.0508	38.6	24	0.22	6.8	15	248	
G02D G02D	9/14/2016	0.0534	34.7	24	0.20	6.6	22	276	
G02D G02D	12/14/2016	0.0552	40.4	24	0.20	6.3	22	266	
G02D G02D	3/8/2017	0.0546	40.4	24	0.19	6.9	18	270	
G02D G02D	6/14/2017	0.0467	33.2	24	0.19	6.3	20	198	
G02D G02D	7/20/2017	0.0407	37.5	23	0.19	6.7	12	264	
G02D G02D		0.0440	40.1	22	0.22	6.9	12	204	
G02D G02D	11/30/2017 6/19/2018	0.0490	33.9	23	0.21	6.7	17	240	
G02D G02D	9/5/2018	0.0468	36.3	23	0.21	6.6	19	252	
G02D G02D	3/27/2019	0.0400	38.7	20	0.10	6.6	20	262	
G02D G02D	9/9/2019	0.0429	40.3	18	0.20	6.5	20	264	
G02D G02D	3/30/2020	0.0449	33.5	20	0.18	6.6	20	204	
_		0.0443	33.5	20	0.10	0.0	22		
Downgradier				-					
G51D	12/3/2015	0.117	39.2	9	0.13	6.2	117	304	
G51D	3/15/2016	0.184	39.7	9	0.10	5.9	145	342	
G51D	6/15/2016	0.213	42.3	7	<0.1	5.8	139	330	
G51D	9/14/2016	0.263	29.6	9	<0.1	5.6	136	360	
G51D	12/14/2016	0.171	30.0	11	<0.1	5.9	101	270	
G51D	3/8/2017	0.309	32.6	8	<0.1	6.2	146	340	
G51D	6/15/2017	0.580	34.0	9	<0.1	5.6	149	340	
G51D	7/20/2017	0.332	31.8	8	<0.1	5.9	140	344	
G51D	11/30/2017	0.302	34.4	8	<0.1	5.6	138	356	
G51D	6/19/2018	0.337	31.1	7	<0.1	5.7	124	324	
G51D	9/5/2018	0.263	29.1	7	<0.1	6.0	134	342	
G51D	3/27/2019	0.778	34.7	6	<0.1	5.7	125	350	
G51D	9/9/2019	0.501	31.3	6	<0.1	5.3	109	320	
G51D	3/30/2020	0.697	31.2	6	<0.1	5.6	130	304	
G52D	12/3/2015	< 0.025	46.6	22	0.25	6.5	65	332	
G52D	3/15/2016	< 0.025	49.1	22	0.26	6.3	99	310	
G52D	6/15/2016	< 0.025	69.2	21	0.25	6.6	88	360	
G52D	9/14/2016	< 0.025	47.6	20	0.26	6.4	84	376	
G52D	12/14/2016	< 0.025	53.4	20	0.25	6.7	82	356	
G52D	3/7/2017	< 0.025	55.0	18	0.24	5.9	115	410	
G52D	6/14/2017	< 0.025	51.0	17	0.24	6.2	112	372	
G52D	7/19/2017	< 0.025	50.7	15	0.27	6.4	108	412	
G52D	11/30/2017	<0.025	54.7	15	0.26	6.0	97	392	

Analytical Results - Appendix III Joppa East Ash Pond

Sample	Date	Boron, total	Calcium, total	Chloride, total	Fluoride, total	рН	Sulfate, total	Total Dissolved Solids
Location	Sampled	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(s.u.)	(mg/L)	(mg/L)
G52D	6/19/2018	<0.025	50.1	15	0.26	6.4	97	388
G52D	9/5/2018	<0.025	49.8	14	0.24	6.3	101	384
G52D	3/27/2019	<0.025	59.8	13	0.28	6.4	81	376
G52D	9/9/2019	<0.025	52.2	14	0.27	6.0	78	370
G52D	3/30/2020	<0.025	48.8	14	0.27	6.4	84	362
G53D	12/3/2015	0.332	62.6	22	0.79	6.8	103	368
G53D	3/15/2016	0.334	50.5	20	0.72	6.7	107	406
G53D	6/15/2016	0.342	47.2	17	0.68	6.6	107	392
G53D	9/14/2016	0.368	44.4	20	0.70	6.5	104	424
G53D	12/14/2016	0.364	44.5	20	0.69	6.8	106	418
G53D	3/8/2017	0.138	23.6	6	0.49	7.2	35	216
G53D	6/15/2017	0.309	38.9	18	0.59	6.6	79	348
G53D	7/20/2017	0.366	40.8	18	0.69	6.8	94	396
G53D	11/30/2017	0.427	44.6	20	0.74	6.6	98	348
G53D	6/19/2018	0.361	37.8	18	0.66	6.6	84	360
G53D	9/5/2018	0.392	40.3	20	0.61	6.8	81	390
G53D	3/27/2019	0.269	30.5	12	0.59	6.6	54	272
G53D	9/9/2019	0.385	42.2	18	0.67	6.2	80	364
G53D	3/30/2020	0.334	34.8	17	0.63	6.7	66	296
G54D	12/3/2015	0.663	103	33	0.38	7.0	191	556
G54D	3/15/2016	0.513	75.2	32	0.38	6.8	176	554
G54D	6/15/2016	0.508	72.8	28	0.34	6.6	160	476
G54D	9/14/2016	0.557	70.4	28	0.34	6.6	149	502
G54D	12/14/2016	0.564	74.3	26	0.32	6.7	144	456
G54D	3/8/2017	0.499	74.1	26	0.30	7.1	131	482
G54D	6/15/2017	0.685	80.5	24	0.32	6.8	170	506
G54D	7/20/2017	0.580	75.7	24	0.32	6.8	151	512
G54D	11/30/2017	0.646	76.2	26	0.33	6.7	136	472
G54D	6/19/2018	0.631	72.7	26	0.34	6.7	146	486
G54D	9/5/2018	0.660	73.6	25	0.30	6.5	152	480
G54D	3/27/2019	1.03	115	22	0.35	6.8	142	510
G54D	9/9/2019	0.614	79.9	<25	0.32	6.4	136	482
G54D	3/30/2020	0.766	84.9	22	0.33	6.8	184	508

Notes:

1. Abbreviations: mg/L - milligrams per liter; s.u. - standard units.

Analytical Results - Appendix IV Joppa East Ash Pond

Sample Location	Date Sampled	Antimony , total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium , total (mg/L)	Cadmium , total (mg/L)	Chromium , total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum , total (mg/L)	Radium- 226 + Radium 228, tot (pCi/L)	Selenium , total (mg/L)	Thallium, total (mg/L)
Background Wells										(
G01D	12/3/2015	<0.001	0.0015	0.254	<0.001	<0.001	0.0047	0.0060	0.26	0.0018	0.0018	< 0.0002	0.0010	0.07	<0.001	<0.001
G01D	3/15/2016	<0.001	0.0026	0.283	<0.001	<0.001	0.0032	0.0136	0.20	0.0012	0.0017	<0.0002	0.0013	0.96	<0.001	<0.001
G01D	6/15/2016	<0.001	0.0018	0.200	<0.001	<0.001	0.0002	0.0128	0.25	< 0.001	0.0015	< 0.0002	0.0010	0.44	<0.001	<0.001
G01D	9/14/2016	< 0.001	0.0021	0.190	< 0.001	< 0.001	0.0031	0.0113	0.26	0.0013	0.0024	< 0.0002	0.0010	0.58	< 0.001	< 0.001
G01D	12/14/2016	< 0.001	0.0012	0.163	< 0.001	< 0.001	0.0036	0.0077	0.24	0.0012	0.0024	< 0.0002	< 0.001	0.40	< 0.001	< 0.001
G01D	3/7/2017	< 0.001	< 0.001	0.155	< 0.001	< 0.001	0.0014	0.0061	0.22	< 0.001	0.0013	< 0.0002	< 0.001	0.24	< 0.001	< 0.001
G01D	6/15/2017	< 0.001	< 0.001	0.140	<0.001	< 0.001	0.0032	0.0047	0.23	0.0013	0.0018	< 0.0002	<0.001	0.93	< 0.001	< 0.001
G01D	7/20/2017	<0.001	0.001	0.140	< 0.001	<0.001	0.0042	0.0035	0.24	0.0014	0.0017	< 0.0002	0.0018	0.41	<0.001	<0.001
G01D	11/30/2017	NA	NA	NA	NA	NA	NA	NA	0.22	NA	NA	NA	NA	NA	NA	NA
G01D	6/19/2018	<0.001	0.0019	0.202	<0.001	<0.001	0.0093	0.0057	0.24	0.0034	0.0038	< 0.0002	<0.0015	1.41	<0.001	<0.002
G01D	9/5/2018	NA	<0.001	0.147	NA	NA	0.0026	0.0022	0.20	<0.001	0.0017	NA	NA	0.37	0.0010	NA
G01D	3/27/2019	<0.001	<0.001	0.129	<0.001	<0.001	0.003	0.0014	0.23	<0.001	0.0015	< 0.0002	<0.0015	0.78	0.0015	<0.002
G01D	9/9/2019	NA	<0.001	0.123	NA	NA	0.0044	0.0014	0.23	0.0012	< 0.003	NA	NA	0.79	0.0011	NA
G01D	3/30/2020	<0.001	0.0011	0.130	<0.001	<0.001	0.0065	0.0018	0.21	0.0019	0.0034	<0.0002	<0.0015	1.44	0.0013	<0.002
G02D	12/3/2015	<0.001	<0.001	0.232	<0.001	<0.001	<0.001	0.0024	0.24	<0.001	0.0011	<0.0002	<0.001	1.10	0.0019	<0.001
G02D	3/15/2016	<0.001	<0.001	0.218	<0.001	<0.001	<0.001	<0.001	0.22	<0.001	<0.001	<0.0002	<0.001	0.47	0.0022	<0.001
G02D	6/15/2016	<0.001	<0.001	0.203	<0.001	<0.001	<0.001	<0.001	0.21	<0.001	0.0012	<0.0002	<0.001	0.63	0.0022	<0.001
G02D	9/14/2016	<0.001	<0.001	0.206	<0.001	<0.001	<0.001	<0.001	0.20	<0.001	0.0013	< 0.0002	<0.001	0.33	0.0033	<0.001
G02D	12/14/2016	<0.001	<0.001	0.224	<0.001	<0.001	0.0057	0.0019	0.19	<0.001	0.0019	<0.0002	<0.001	0.40	0.0039	<0.001
G02D	3/8/2017	<0.001	<0.001	0.211	<0.001	<0.001	<0.001	<0.001	0.19	<0.001	<0.001	<0.0002	<0.001	1.06	0.0024	<0.001
G02D	6/14/2017	<0.001	<0.001	0.192	<0.001	<0.001	<0.001	<0.001	0.19	<0.001	0.0013	<0.0002	<0.001	0.63	0.0023	<0.001
G02D	7/20/2017	<0.001	<0.001	0.211	<0.001	<0.001	0.0016	<0.001	0.22	<0.001	<0.001	<0.0002	<0.001	1.33	0.0016	<0.001
G02D	11/30/2017	NA	NA	NA	NA	NA	NA	NA	0.21	NA	NA	NA	NA	NA	NA	NA
G02D	6/19/2018	<0.001	<0.001	0.245	<0.001	<0.001	<0.0015	<0.001	0.21	<0.001	<0.0015	<0.0002	<0.0015	0.92	0.0023	<0.002
G02D	9/5/2018	NA	<0.001	0.209	NA	NA	<0.0015	<0.001	0.18	< 0.001	<0.0015	NA	NA	0.46	0.002	NA
G02D	3/27/2019	<0.001	< 0.001	0.235	<0.001	< 0.001	0.0026	<0.001	0.20	<0.001	<0.0015	< 0.0002	<0.0015	0.12	0.003	<0.002
G02D	9/9/2019	NA	< 0.001	0.208	NA	NA	< 0.0015	< 0.001	0.21	< 0.001	< 0.003	NA	NA	0.49	0.0021	NA
G02D	3/30/2020	<0.001	<0.001	0.202	<0.001	<0.001	<0.0015	<0.001	0.18	<0.001	<0.003	<0.0002	<0.0015	0.79	0.0035	<0.002
Downgradient W																
G51D	12/3/2015	<0.001	<0.001	0.129	<0.001	<0.001	<0.001	0.0141	0.13	<0.001	0.0035	<0.0002	<0.001	0.02	0.0024	<0.001
G51D	3/15/2016	<0.001	<0.001	0.0702	<0.001	<0.001	0.0014	0.0249	0.10	<0.001	0.0048	<0.0002	<0.001	0.69	0.0019	<0.001
G51D	6/15/2016	<0.001	<0.001	0.0628	<0.001	<0.001	<0.001	0.0198	<0.1	<0.001	0.0059	<0.0002	<0.001	0.43	0.0028	<0.001
G51D	9/14/2016	<0.001	<0.001	0.0536	<0.001	<0.001	<0.001	0.0110	<0.1	<0.001	0.0052	<0.0002	<0.001	0.80	0.0031	<0.001
G51D	12/14/2016	<0.001	<0.001	0.0459	<0.001	<0.001	<0.001	0.0119	<0.1	<0.001	0.0050	<0.0002	<0.001	0.29	0.0031	<0.001
G51D	3/8/2017	<0.001	<0.001	0.0493	<0.001	< 0.001	<0.001	0.0082	<0.1	<0.001	0.0045	< 0.0002	<0.001	0.52	0.0033	<0.001
G51D	6/15/2017	< 0.001	<0.001	0.0442	< 0.001	< 0.001	< 0.001	0.0052	<0.1	< 0.001	0.0058	< 0.0002	<0.001	0.56	0.0039	< 0.001
G51D	7/20/2017	< 0.001	<0.001	0.0462	<0.001	< 0.001	<0.001	0.0055	<0.1	<0.001	0.0047	< 0.0002	<0.001	1.68	0.0035	< 0.001
G51D	11/30/2017	NA	NA	NA	NA	NA	NA	NA	<0.1	NA	NA	NA	NA	NA	NA	NA
G51D	6/19/2018	<0.001	< 0.001	0.0756	<0.001	< 0.001	<0.0015	0.0038	<0.1	< 0.001	0.0070	<0.0002	<0.0015	1.22	0.0035	<0.002
G51D	9/5/2018	NA	< 0.001	0.0395	#N/A	#N/A	< 0.0015	0.0043	<0.1	< 0.001	0.0054	NA	NA	0.42	0.0036	NA
G51D	3/27/2019	<0.001	<0.001	0.0495	<0.001	<0.001	0.0016	0.0026	<0.1	< 0.001	0.0059	<0.0002	<0.0015	0.23	0.005	<0.002
G51D	9/9/2019	NA	< 0.001	0.0377	#N/A	#N/A	<0.0015	0.0017	<0.1	<0.001	0.0057	NA	NA	0.36	0.0042	NA
G51D	3/30/2020	<0.001	<0.001	0.0445	<0.001	<0.001	0.0019	0.0024	<0.1	<0.001	0.0065	<0.0002	<0.0015	0.90	0.0048	<0.002

Analytical Results - Appendix IV Joppa East Ash Pond

Sample	Date	Antimony , total	Arsenic, total	Barium, total	Beryllium , total	Cadmium , total	Chromium , total	Cobalt, total	Fluoride, total	Lead, total	Lithium, total	Mercury, total	Molybdenum , total	Radium- 226 + Radium 228, tot	Selenium , total	Thallium total
Location	Sampled	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(pCi/L)	(mg/L)	(mg/L)
G52D	12/3/2015	<0.001	0.0032	0.318	<0.001	<0.001	<0.001	0.0056	0.25	<0.001	0.0026	<0.0002	0.0017	0.31	<0.001	<0.001
G52D	3/15/2016	<0.001	0.0051	0.345	<0.001	<0.001	<0.001	0.0064	0.26	<0.001	0.0023	<0.0002	<0.001	1.16	<0.001	<0.001
G52D	6/15/2016	<0.001	0.0072	0.506	<0.001	<0.001	<0.001	0.0093	0.25	<0.001	0.0040	<0.0002	<0.001	2.18	<0.001	<0.001
G52D	9/14/2016	<0.001	0.0043	0.362	<0.001	<0.001	<0.001	0.0063	0.26	<0.001	0.0027	<0.0002	0.0017	1.81	<0.001	<0.001
G52D	12/14/2016	<0.001	0.0030	0.356	<0.001	<0.001	<0.001	0.0030	0.25	<0.001	0.0036	<0.0002	0.0024	1.24	<0.001	<0.001
G52D	3/7/2017	<0.001	0.0066	0.358	<0.001	<0.001	<0.001	0.0072	0.24	<0.001	0.0024	<0.0002	<0.001	0.80	<0.001	<0.001
G52D	6/14/2017	<0.001	0.0054	0.289	<0.001	<0.001	<0.001	0.0062	0.24	<0.001	0.0030	<0.0002	<0.001	1.28	<0.001	<0.001
G52D	7/19/2017	<0.001	0.0016	0.293	<0.001	<0.001	<0.001	0.0013	0.27	<0.001	0.0028	<0.0002	0.0011	0.80	<0.001	<0.001
G52D	11/30/2017	NA	NA	NA	NA	NA	NA	NA	0.26	NA	NA	NA	NA	NA	NA	NA
G52D	6/19/2018	<0.001	0.0025	0.340	<0.001	<0.001	<0.0015	0.0045	0.26	<0.001	0.0035	<0.0002	<0.0015	1.64	<0.001	<0.002
G52D	9/5/2018	NA	0.0014	0.275	NA	NA	<0.0015	0.0019	0.24	<0.001	0.0032	NA	NA	0.68	<0.001	NA
G52D	3/27/2019	<0.001	0.0064	0.271	<0.001	<0.001	<0.0015	0.0069	0.28	<0.001	0.0028	<0.0002	<0.0015	0.58	<0.001	<0.002
G52D	9/9/2019	NA	0.0021	0.254	NA	NA	<0.0015	0.0022	0.27	<0.001	< 0.003	NA	NA	1.54	<0.001	NA
G52D	3/30/2020	<0.001	0.0017	0.254	<0.001	<0.001	<0.0015	0.0033	0.27	<0.001	< 0.003	<0.0002	<0.0015	1.30	<0.001	<0.002
G53D	12/3/2015	<0.001	<0.001	0.353	<0.001	<0.001	0.0017	0.0087	0.79	<0.001	0.0020	<0.0002	0.0013	0.28	<0.001	<0.001
G53D	3/15/2016	<0.001	<0.001	0.279	<0.001	<0.001	<0.001	0.0087	0.72	<0.001	0.0015	< 0.0002	0.0012	1.24	< 0.001	<0.001
G53D	6/15/2016	<0.001	<0.001	0.207	<0.001	<0.001	<0.001	0.0059	0.68	<0.001	0.0017	<0.0002	<0.001	1.11	<0.001	<0.001
G53D	9/14/2016	<0.001	<0.001	0.191	< 0.001	<0.001	<0.001	0.002	0.70	<0.001	0.0016	< 0.0002	<0.001	0.10	< 0.001	<0.001
G53D	12/14/2016	<0.001	<0.001	0.169	< 0.001	<0.001	<0.001	0.0029	0.69	<0.001	0.0018	< 0.0002	<0.001	0.39	<0.001	<0.001
G53D	3/8/2017	<0.001	<0.001	0.109	<0.001	<0.001	0.0018	0.0027	0.49	<0.001	0.0021	< 0.0002	<0.001	0.08	<0.001	<0.001
G53D	6/15/2017	<0.001	<0.001	0.172	<0.001	<0.001	<0.001	<0.001	0.59	<0.001	0.0022	< 0.0002	<0.001	0.16	<0.001	<0.001
G53D	7/20/2017	<0.001	<0.001	0.165	<0.001	<0.001	<0.001	0.0011	0.69	<0.001	0.0015	< 0.0002	<0.001	1.25	< 0.001	<0.001
G53D	11/30/2017	NA	NA	NA	NA	NA	NA	NA	0.74	NA	NA	NA	NA	NA	NA	NA
G53D	6/19/2018	<0.001	<0.001	0.176	<0.001	<0.001	<0.0015	<0.001	0.66	<0.001	0.0019	< 0.0002	<0.0015	0.77	< 0.001	< 0.002
G53D	9/5/2018	NA	<0.001	0.133	NA	NA	<0.0015	0.0016	0.61	<0.001	0.0018	NA	NA	0.55	<0.001	NA
G53D	3/27/2019	<0.001	<0.001	0.101	<0.001	<0.001	<0.0015	<0.001	0.59	<0.001	<0.0015	< 0.0002	<0.0015	0.17	<0.001	< 0.002
G53D	9/9/2019	NA	<0.001	0.128	NA	NA	<0.0015	0.002	0.67	<0.001	< 0.003	NA	NA	0.03	<0.001	NA
G53D	3/30/2020	<0.001	<0.001	0.109	<0.001	<0.001	<0.0015	<0.001	0.63	<0.001	< 0.003	<0.0002	<0.0015	1.32	<0.001	< 0.002
G54D	12/3/2015	<0.001	0.0020	0.115	<0.001	<0.001	0.0016	0.0268	0.38	<0.001	0.0069	<0.0002	<0.001	0.20	<0.001	<0.001
G54D	3/15/2016	<0.001	0.0025	0.106	< 0.001	<0.001	0.0030	0.0183	0.38	<0.001	0.0078	< 0.0002	0.0010	0.354	< 0.001	<0.001
G54D	6/15/2016	<0.001	0.0020	0.114	<0.001	<0.001	<0.001	0.0158	0.34	<0.001	0.0068	< 0.0002	<0.001	1.02	< 0.001	<0.001
G54D	9/14/2016	<0.001	0.0026	0.134	< 0.001	<0.001	<0.001	0.0167	0.34	<0.001	0.0062	< 0.0002	<0.001	0.39	<0.001	<0.001
G54D	12/14/2016	<0.001	0.0033	0.138	< 0.001	<0.001	<0.001	0.0178	0.32	<0.001	0.0061	< 0.0002	<0.001	1.05	<0.001	< 0.001
G54D	3/8/2017	<0.001	0.0025	0.132	<0.001	<0.001	<0.001	0.017	0.30	<0.001	0.0048	< 0.0002	<0.001	0.68	< 0.001	<0.001
G54D	6/15/2017	< 0.001	<0.001	0.105	< 0.001	<0.001	0.0018	0.016	0.32	<0.001	0.0047	< 0.0002	< 0.001	1.67	< 0.001	<0.001
G54D	7/20/2017	<0.001	0.0012	0.127	<0.001	<0.001	0.0017	0.0139	0.32	<0.001	0.0044	<0.0002	<0.001	0.32	<0.001	<0.001
G54D	11/30/2017	NA	NA	NA	NA	NA	NA	NA	0.33	NA	NA	NA	NA	NA	NA	NA
G54D	6/19/2018	<0.001	0.0019	0.196	<0.001	<0.001	0.0019	0.0134	0.34	<0.001	0.0060	<0.0002	<0.0015	1.00	<0.001	<0.002
G54D	9/5/2018	NA	0.0010	0.131	NA	NA	0.0020	0.0109	0.30	< 0.001	0.0046	NA	NA	1.32	< 0.001	NA
G54D	3/27/2019	< 0.001	0.0011	0.120	< 0.001	< 0.001	<0.0015	0.0138	0.35	< 0.001	0.0037	< 0.0002	< 0.0015	0.42	< 0.001	< 0.002
G54D	9/9/2019	NA	<0.001	0.128	NA	NA	<0.0015	0.0117	0.32	< 0.001	0.0037	NA	NA	0.84	< 0.001	NA
G54D	3/30/2020	< 0.001	< 0.001	0.105	<0.001	<0.001	<0.0015	0.013	0.33	< 0.001	0.0036	<0.0002	< 0.0015	0.89	< 0.001	<0.002

1. Abbreviations: mg/L - milligrams per liter; NA - not analyzed; pCi/L - picocurie per liter;

ATTACHMENT 6 – SITE HYDROGEOLOGY AND STRATIGRAPHIC CROSS-SECTIONS OF THE SITE

CONCEPTUAL SITE MODEL AND DESCRIPTION OF SITE HYDROGEOLOGY (EAST ASH POND)

The Joppa Power Station (Power Station) conceptual site model (CSM) and Description of Site Hydrogeology for the Joppa East Ash Pond (EAP), located in Joppa, Illinois are described in the following sections.

REGIONAL SETTING

The Power Station is located west of the Village of Joppa in Massac County, Illinois, northeast of the Ohio River in Section 14, Township 15 South, Range 3 East of the 3rd Principal Meridian. The EAP is located in the west half of Section 14 directly north of the Power Station. The Power Station property is bordered by LaFarge North America cement plant to the west, Trunkline Gas Company-Joppa Compressor Station to the north, the Village of Joppa to the east and the Ohio River to the south.

The EAP lies adjacent to and north to northeast of the Ohio River at the southern boundary of the Illinois Basin and the northern edge of the Mississippi Embayment, a relatively low lying area that is part of the Coastal Plain Physiographic Province (Leighton, 1948). Based on stack-unit maps prepared by the Illinois State Geological Survey (Berg and Kempton, 1987) the area is characterized by less than 20 feet of silty and clayey diamictons overlying Cretaceous-age sediments, silts, sands, etc., between depths of 20 to 50 feet. However, in some areas along the Ohio River, the predominant unlithified materials are Quaternary-age sand and gravel outwash deposits belonging to the Henry Formation. The unlithified materials rest on Mississippian-age bedrock. The bedrock dips gently northward toward the center of the Illinois Basin.

SITE GEOLOGY

Previous investigations and reports at Joppa East Ash Pond indicate the surface impoundment is underlain by more than 50-feet of clay-rich deposits (predominantly clay and silty clay deposits with some minor intervals of sandy clay deposits) of the Equality and Metropolis Formations (NRT, 2013).

SITE HYDROGEOLOGY

The CCR groundwater monitoring system consists of six monitoring wells installed in the uppermost aquifer and adjacent to the EAP (G01D, G02D, G51D, G52D, G53D, G54D) (see Monitoring Well Location Map, and Well Construction Diagrams and Drilling Logs attached to this demonstration). The unit utilizes two background monitoring wells (G01D and G02D) as part of the CCR groundwater monitoring system.

The uppermost aquifer consists of intermittent unlithified silty sand deposits within the McNairy Formation. The McNairy Formation was described as a hydrostratigraphic unit with greater permeability than the overlying clay-rich deposits of the Equality and Metropolis Formations in the Phase I Hydrogeologic Site Assessment Report (NRT, 2013). The report indicated the overlying hydrostratigraphic unit (inclusive of both the Equality and Metropolis Formations) is a confining unit, where the geometric mean hydraulic conductivity of the unit was 5.9 x 10-6 centimeters per second (cm/s). No known wells in the area utilize the Equality and Metropolis Formations for groundwater and most wells obtain groundwater from sands and gravels of the McNairy Formation or underlying Mississippian-age limestone bedrock. The McNairy Formation was also identified as the uppermost aquifer in the vicinity of Joppa Landfill, located northwest of the EAP (Hanson, 2009). The uppermost aquifer is laterally continuous across the Power Station and is approximately 85 feet thick in the vicinity of the EAP.

The lower limit of the uppermost aquifer (McNairy Formation) is the Mississippian-age Salem Limestone bedrock. The Mississippian-age bedrock in the vicinity of the EAP is a useable groundwater



resource (NRT,2013). The fractured limestone bedrock has widely variable transmissivities and is estimated to have a thickness of approximately 200 to 500 feet. Bedrock was intercepted at an elevation of approximately 210 feet MSL at plant well 4 located south of the EAP. Bedrock was not encountered in borings performed at the EAP.

Hydraulic Conductivity

Falling/rising head tests were completed in wells screened in the unlithified material of the McNairy Formation (uppermost aquifer) as part of the supplemental site characterization activities completed by NRT in 2017. The single-well falling/rising head tests indicate the McNairy Formation has a moderate hydraulic conductivity ranging from 2.4 x 10-5 to 9.9 x 10-4 cm/s, with the exception of monitoring well location G52D, which exhibited a hydraulic conductivity of 7.1 x 10-8 cm/s. The geometric mean of hydraulic conductivities in the McNairy Formation was 2.4 x 10-4 cm/s. The effective porosity of this aquifer likely ranges significantly in magnitude due to its variable composition. The effective porosity of the aquifer was estimated (20%) from literature values (Smith and Wheatcraft, 1993) to calculate the groundwater velocity.

Groundwater Elevations, Flow Direction and Velocity

Average measured groundwater elevations range from approximately 319 ft MSL in the northern portion of the EAP (upgradient, G01D and G02D), to approximately 312 ft MSL in southern portion of the EAP (downgradient, G54D). Groundwater elevations at downgradient well G52D, located southeast of the EAP, are typically more than 5 feet higher than nearby downgradient well G54D, located southwest of the EAP. A significant portion of the well screen at monitoring well G52D is across primarily clay and silt materials which may influence the hydraulic heads measured at the monitoring well location. Screened materials at G52D were also shown to have a decreased hydraulic conductivity when compared to other monitoring well locations which may influence the measured hydraulic heads.

Groundwater elevations vary seasonally and may fluctuate by about 10 feet. Slight seasonal variation in groundwater flow directions ranging from southeast to southwest are also observed, however, the major component of groundwater flow direction is consistently south toward the Ohio River, which is the primary discharge area in the vicinity of the EAP (NRT, 2013).

Horizontal hydraulic gradients are moderate across the EAP well network and ranged from 0.002 feet per feet (ft/ft) in December 2016 to 0.003 ft/ft in June 2017. Horizontal hydraulic gradients upgradient of the EAP (measured from monitoring wells G01D and G02D) appeared to be slightly lower.

Groundwater flow velocity at the EAP ranged from 0.003 to 0.01 feet per day (ft/day) in December 2016 and June 2017. In December 2016, groundwater flow velocity was 0.008 ft/day as groundwater flowed from northwest to southeast across the central portion of the EAP, while in June 2017 groundwater flow velocity was 0.01 ft/day. Near upgradient monitoring wells G01D and G02D, groundwater flow velocity was 0.003 ft/day in December 2016 and 0.009 ft/day in June 2017.

REFERENCES

Berg, R.C., and J.P. Kempton, 1987, Stack-Unit Mapping of Geologic Materials in Illinois to a Depth of 15 Meters: Illinois State Geological Survey, Circular 542, 23 p.

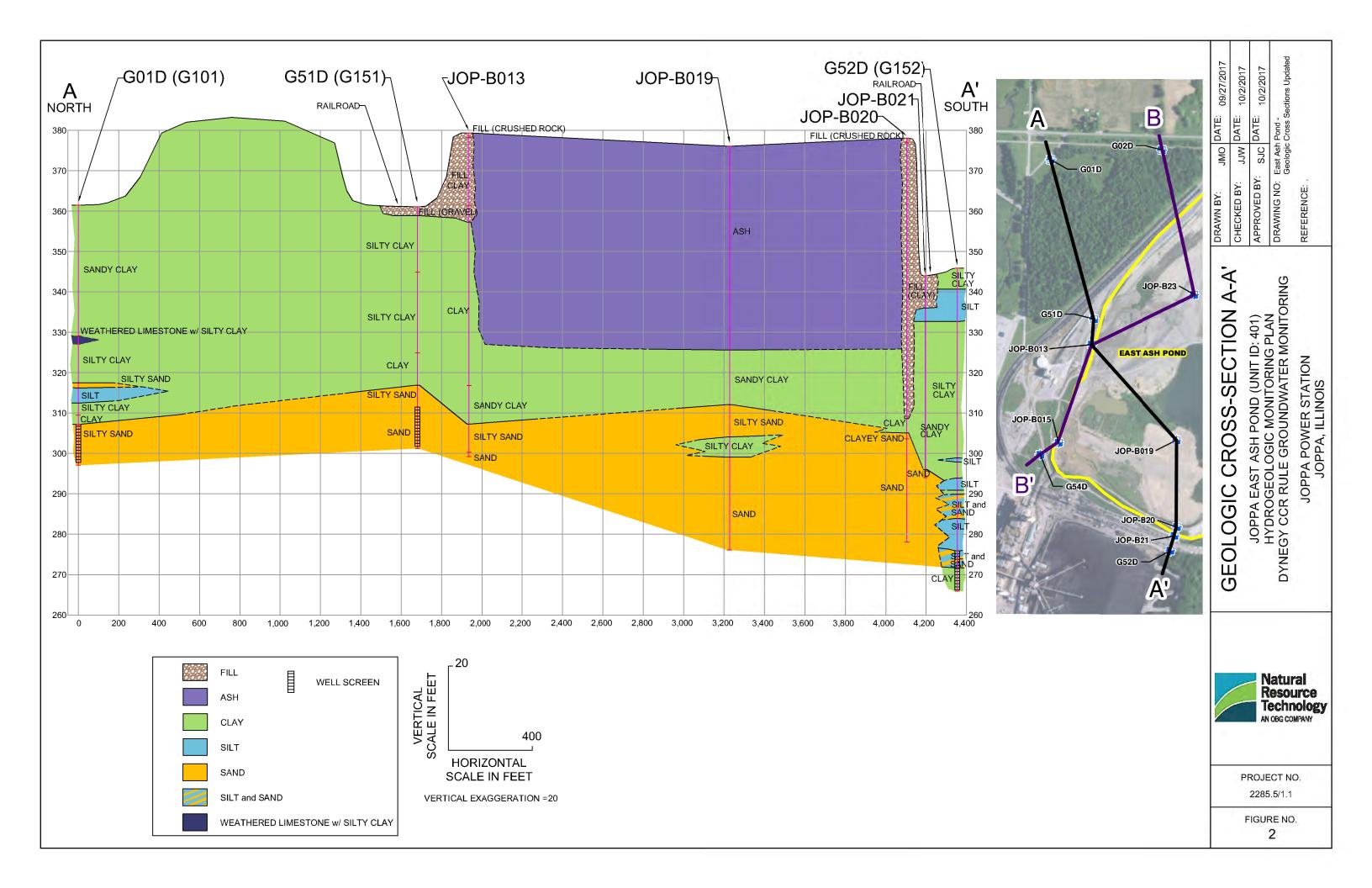
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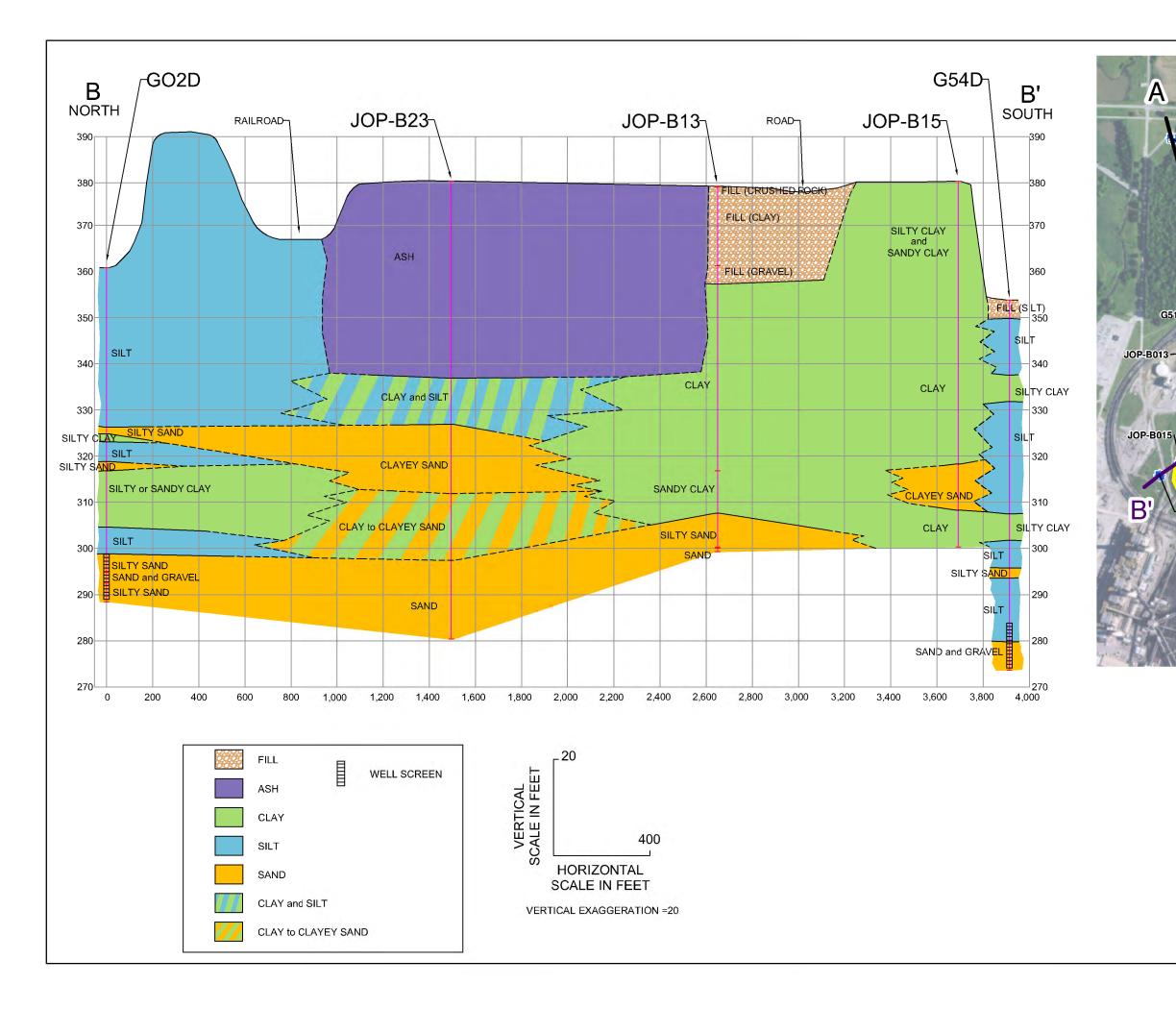


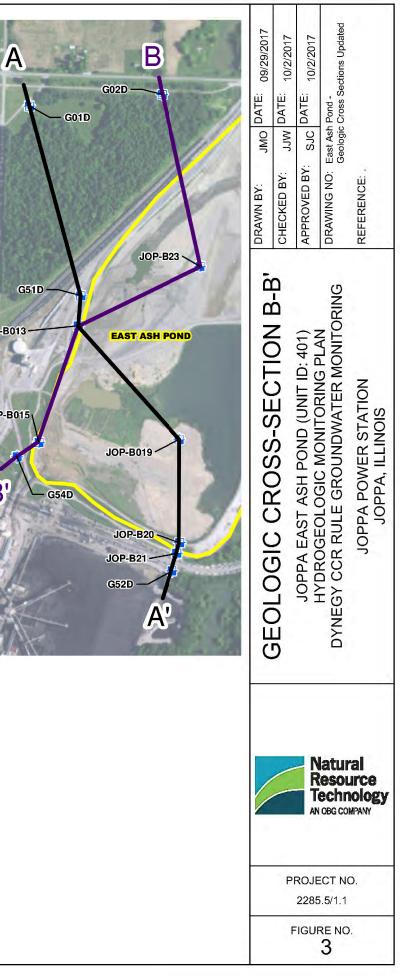
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ATTACHMENT 7 – STRUCTURAL STABILITY ASSESSMENT



Submitted to Electric Energy, Inc. 2200 Portland Road Metropolis, IL 62960 Submitted by AECOM 1001 Highlands Plaza Drive West Suite 300 St. Louis, MO 63110

October 2016

CCR Rule Report: Initial Structural Stability Assessment For East Ash Pond At Joppa Power Station

1 Introduction

This Coal Combustion Residual (CCR) Rule Report documents that the East Ash Pond at the Electric Energy, Inc. (EEI) Joppa Power Station meets the structural stability assessment requirements specified in 40 Code of Federal Regulations (CFR) §257.73(d), except as noted herein. The East Ash Pond is located near Joppa, Illinois in Massac County, approximately 0.1 miles northeast of the Joppa Power Station. The East Ash Pond serves as the wet impoundment basin for CCRs produced by the Joppa Power Station.

The East Ash Pond is an existing CCR surface impoundment as defined by 40 CFR §257.53. The CCR Rule requires that an initial structural stability assessment for an existing CCR surface impoundment be completed by October 17, 2016. In general, the initial structural stability assessment must document that the design, construction, operation, and maintenance of the CCR unit is consistent with recognized and generally accepted good engineering practices.

The owner or operator of the CCR unit must obtain a certification from a qualified professional engineer stating that the initial structural stability assessment was conducted in accordance with the requirements of 40 CFR §257.73(d). The owner or operator must prepare a periodic structural stability assessment every five years.

2 Initial Structural Stability Assessment

40 CFR §257.73(d)(1)

The owner or operator of the CCR unit must conduct initial and periodic structural stability assessments and document whether the design, construction, operation, and maintenance of the CCR unit is consistent with recognized and generally accepted good engineering practices for the maximum volume of CCR and CCR wastewater which can be impounded therein. The assessment must, at a minimum, document whether the CCR unit has been designed, constructed, operated, and maintained with [the standards in (d)(1)(i)-(vii)].

An initial structural stability assessment has been performed to document that the design, construction, operation and maintenance of the East Ash Pond is consistent with recognized and generally accepted good engineering practices. The results of the structural stability assessment are discussed in the following sections. Based on the assessment and its results, the design, construction, operation, and maintenance of the Joppa East Ash Pond were found to be consistent with recognized and generally accepted good engineering practices, and meet the standards in \$257.73(d)(1)(i)-(vii), except as noted herein.

2.1 Foundations and Abutments (§257.73(d)(1)(i))

CCR unit designed, constructed, operated, and maintained with stable foundations and abutments.

The stability of the foundations was evaluated using soil data from field investigations and reviewing design drawings, operational and maintenance procedures, and conditions observed in the field by AECOM. Additionally, slope stability analyses were performed to evaluate slip surfaces passing through the foundations. The East Ash Pond is a ring dike structure and does not have abutments.

The foundation consists of medium stiff to stiff clay overlying medium dense to dense sand, which indicates stable foundations. Ash placed before 1973 was identified below the compacted embankment of the dike and above the naturally occurring foundation materials near the southeast corner of the East Ash Pond embankment; however this condition was modified by installing a Deep Soil Mixed (DMM), wet soil cement mixed method, zone in 2016. Slope stability analyses exceed the criteria listed in §257.73(e)(1) for slip surfaces passing through the foundation. The slope stability analyses are discussed in the *CCR Rule Report: Initial Safety Factor Assessment for East Ash Pond at Joppa Power Station* (October 2016). Additional slope stability analyses were performed to evaluate the effects of liquefaction in the foundation and were found to satisfy the criteria in §257.73(e)(1)(iv) applicable to dikes. A review of information about operations and maintenance as well as current and past performance of the dikes has determined appropriate processes are in place for continued operational performance.

Based on the conditions observed by AECOM, the East Ash Pond was designed and constructed with stable foundations. Any issues related to the stability of the foundation are addressed during operations and maintenance; therefore, the East Ash Pond meets the requirements in §257.73(d)(1)(i).

2.2 Slope Protection (§257.73(d)(1)(ii))

CCR unit designed, constructed, operated, and maintained with adequate slope protection to protect against surface erosion, wave action and adverse effects of sudden drawdown.

The adequacy of slope protection was evaluated by reviewing design drawings, information about operations and maintenance, and conditions observed in the field by AECOM.

Based on this evaluation, adequate slope protection was designed and constructed at the East Ash Pond. No evidence of significant areas of erosion or wave action was observed. The exterior dike slopes are covered with crushed stone and vegetation for slope protection. EEI regularly maintains the slopes, including repairing observed surface erosion and addressing areas of poor vegetation growth, as required. As the exterior slopes are not adjacent to a downstream water body, they are not susceptible to wave action or sudden drawdown. AECOM observed the vegetation to be adequately protecting against surface erosion.

The interior dike slopes have a 1.5H:1V orientation and are covered with vegetation, stacked CCRs, and some limited areas of crushed stone. Sudden drawdown conditions are not expected to occur due to the characteristics of the outfall structure at the East Ash Pond. EEI regularly maintains the interior slopes, including repairing observed surface erosion and addressing areas of poor vegetation growth, as required. AECOM observed the vegetation to be adequately protecting against surface erosion and wave action. Therefore, the East Ash Pond meets the requirements in §257.73(d)(1)(ii).

2.3 Dike Compaction (§257.73(d)(1)(iii))

CCR unit designed, constructed, operated, and maintained with dikes mechanically compacted to a density sufficient to withstand the range of loading conditions in the CCR unit.

The density of the dike materials was evaluated using soil strength data from field investigations and reviewing design drawings, information about operations and maintenance, and conditions observed in the field by AECOM. Additionally, slope stability analyses were performed to evaluate slip surfaces passing through the dike over the range of expected loading conditions as defined within §257.73(e)(1).

Based on this evaluation, the dike consists of medium stiff to stiff material, which is indicative of properly mechanically compacted dikes. Slope stability analyses exceed the criteria listed in §257.73(e)(1) for slip surfaces passing through the dike. The slope stability analyses are discussed in the *CCR Rule Report: Initial Safety Factor Assessment for East Ash Pond at Joppa Power Station* (October 2016); therefore, the original design and construction of the East Ash Pond included sufficient dike compaction. Operational and maintenance procedures are in place to identify and mitigate deficiencies in order to maintain sufficient compaction and density of the dikes to withstand the range of loading conditions. Therefore, the East Ash Pond meets the requirements in §257.73(d)(1)(iii).

2.4 Vegetated Slopes (§257.73(d)(1)(iv))¹

CCR unit designed, constructed, operated, and maintained with vegetated slopes of dikes and surrounding areas, except for slopes which have an alternate form or forms of slope protection.

The adequacy of slope vegetation was evaluated by reviewing design drawings, information about operations and maintenance, and conditions observed in the field by AECOM.

Based on this evaluation, the vegetation on the exterior slopes is adequate as no substantial bare or overgrown areas were observed. The exterior slopes were vegetated or covered with crushed stone and the interior slopes were covered with vegetation, crushed stone, or mechanically-stacked CCR which are alternate forms of slope protection. Therefore, the original design and construction of the East Ash Pond included adequate vegetation of the dikes and surrounding areas. Adequate operational and maintenance practices are in place to regularly manage vegetation growth, including mowing and seeding any bare areas, as evidenced by the conditions observed by AECOM. Therefore, the East Ash Pond meets the requirements in §257.73(d)(1)(iv).

¹ As modified by court order issued June 14, 2016, Utility Solid Waste Activities Group v. EPA, D.C. Cir. No. 15-1219 (order granting remand and vacatur of specific regulatory provisions).

2.5 Spillways (§257.73(d)(1)(v))

CCR unit designed, constructed, operated, and maintained with a single spillway or a combination of spillways configured as specified in [paragraph (A) and (B)]:

- (A) All spillways must be either:
 - (1) of non-erodible construction and designed to carry sustained flows; or

(2) earth- or grass-lined and designed to carry short-term, infrequent flows at non-erosive velocities where sustained flows are not expected.

- (B) The combined capacity of all spillways must adequately manage flow during and following the peak discharge from a: (1) Probable maximum flood (PMF) for a high hazard potential CCR surface impoundment; or
 - (2) 1000-year flood for a significant hazard potential CCR surface impoundment; or
 - (3) 100-year flood for a low hazard potential CCR surface impoundment.

The spillways were evaluated using design drawings, information about operations and maintenance, and conditions observed in the field by AECOM. Additionally, hydrologic and hydraulic analyses were completed to evaluate the capacity of the spillway relative to inflow estimated for the probable maximum flood event for the high hazard potential East Ash Pond.

Two spillway structures are present at the East Ash Pond, including a ductile iron pipe and high-density polyethylene (HDPE) spillway pipe and a reinforced concrete pipe (RCP) spillway. The ductile iron, HDPE, and reinforced concrete are non-erodible materials designed to carry sustained flows. The capacity of the spillways was evaluated using hydrologic and hydraulic analysis performed per §257.82(a). The analysis found that the spillways can adequately manage flow during peak discharge resulting from the probable maximum flood event without overtopping of the embankments. The hydrologic and hydraulic analyses are discussed in the *CCR Rule Report: Initial Inflow Design Flood Control System Plan for East Ash Pond at Joppa Power Station* (October 2016). Therefore, the East Ash Pond meets the requirements in §257.73(d)(1)(v).

2.6 Stability and Structural Integrity of Hydraulic Structures (§257.73(d)(1)(vi))

CCR unit designed, constructed, operated, and maintained with hydraulic structures underlying the base of the CCR unit or passing through the dike of the CCR unit that maintain structural integrity and are free of significant deterioration, deformation, distortion, bedding deficiencies, sedimentation, and debris which may negatively affect the operation of the hydraulic structure.

Two hydraulic structures pass through the dike of the East Ash Pond: the south-sub-basin 26-inch diameter HDPE spillway pipe and the north sub-basin 48-inch RCP spillway pipe. The stability and structural integrity of both pipes were evaluated using design drawings, information about operations and maintenance, inspections, and conditions observed in the field by AECOM. No other hydraulic structures are known to pass through the dike of or underlie the base of the East Ash Pond.

Closed circuit television (CCTV) inspections of both pipes were attempted in 2016. The 48-inch RCP was successfully visually inspected and noted to contain 12-inches of cemented CCR materials. However, the pipe was free of significant deterioration, deformation, distortion, and bedding deficiencies that may negatively affect the operation of the hydraulic structure. The presence of the cemented CCR materials was accounted for in the hydrologic and hydraulic analyses discussed in the *CCR Rule Report: Initial Inflow Design Flood Control System Plan for East Ash Pond at Joppa Power Station* (October 2016), and were not found sufficient enough to negatively affect the operation of the hydraulic structure. The inspection of the HDPE spillway pipe could not be fully completed due to access issues that prevented an inspection of the entire pipe. However, the evaluation of design drawings, operational and maintenance procedures, and conditions observed in the field did not identify any issues with the HDPE spillway pipe.

Based on this evaluation, all East Ash Pond hydraulic structures cannot be certified to meet the requirements of §257.73(d)(1)(vi) because a complete CCTV inspection of the 26-inch HDPE pipe has not yet been performed, thus, precluding completion of the evaluation of the stability and structural integrity of that pipe. In accordance

2.7 Downstream Slope Inundation/Stability (§257.73(d)(1)(vii))

CCR unit designed, constructed, operated, and maintained with, for CCR units with downstream slopes which can be inundated by the pool of an adjacent water body, such as a river, stream or lake, downstream slopes that maintain structural stability during low pool of the adjacent water body or sudden drawdown of the adjacent water body.

The structural stability of the downstream slope of the East Ash Pond was evaluated by comparing the location of the East Pond relative to published flood maps for the area. The East Ash Pond is located outside of the 100-year flood zone shown on the FEMA Federal Insurance Rate Map (FIRM) map for Massac County, Illinois and the village of Joppa, Illinois. Therefore, adjacent water bodies that could potentially inundate the downstream slopes of the East Ash Pond are not present. Based on this evaluation, the requirements in §257.73(d)(1)(vii) are not applicable to the East Ash Pond, as inundation of the downstream slopes is not expected to occur during 100-year or lesser flood conditions.

3 Certification Statement

CCR Unit: Electric Energy, Inc.; Joppa Power Station; East Ash Pond

I, Victor A. Modeer, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief that the information contained in this CCR Rule Report, and the underlying data in the operating record, has been prepared in accordance with the accepted practice of engineering. I certify, for the above-referenced CCR Unit, that the initial structural stability assessment dated October 14, 2016 was conducted in accordance with the requirements of 40 CFR §257.73.

Modeer Ur

Printed Name

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Date



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1001 Highlands Plaza Drive Wes Suite 300 St. Louis, MO 63110 1-314-429-0100 **ATTACHMENT 8 – SAFETY FACTOR ASSESSMENT**



Submitted to Electric Energy, Inc. 2200 Portland Road Metropolis, IL 62960 Submitted by AECOM 1001 Highlands Plaza Drive West Suite 300 St. Louis, MO 63110

October 2016

CCR Rule Report: Initial Safety Factor Assessment For East Ash Pond At Joppa Power Station

1 Introduction

This Coal Combustion Residual (CCR) Rule Report documents that the East Ash Pond at the Electric Energy, Inc. (EEI) Joppa Power Station meets the safety factor assessment requirements specified in 40 Code of Federal Regulations (CFR) §257.73(e). The East Ash Pond is located near Joppa, Illinois in Massac County, approximately 0.1 miles northeast of the Joppa Power Station. The East Ash Pond serves as the ash impoundment basin for CCRs produced at the Joppa Power Station.

The East Ash Pond is an existing CCR surface impoundment as defined by 40 CFR §257.53. The CCR Rule requires that the initial safety factor assessment for an existing CCR surface impoundment be completed by October 17, 2016.

The owner or operator of the CCR unit must obtain a certification from a qualified professional engineer stating that the initial safety factor assessment meets the requirements of 40 CFR § 257.73(e). The owner or operator must prepare a safety factor assessment every five years.

2 Initial Safety Factor Assessment

40 CFR §257.73(e)(1)

The owner or operator must conduct initial and periodic safety factor assessments for each CCR unit and document whether the calculated factors of safety for each CCR unit achieve the minimum safety factors specified in (e)(1)(i) through (iv) of this section for the critical cross section of the embankment. The critical cross section is the cross section anticipated to be the most susceptible of all cross sections to structural failure based on appropriate engineering considerations, including loading conditions. The safety factor assessments must be supported by appropriate engineering calculations.

(i) The calculated static factor of safety under the long-term, maximum storage pool loading condition must equal or exceed 1.50.

(ii) The calculated static factor of safety under the maximum surcharge pool loading condition must equal or exceed 1.40.

(iii) The calculated seismic factor of safety must equal or exceed 1.00.

(iv) For dikes constructed of soils that have susceptibility to liquefaction, the calculated liquefaction factor of safety must equal or exceed 1.20.

A geotechnical investigation program and stability analyses were performed to evaluate the design, performance, and condition of the earthen dikes of the East Ash Pond. The exploration consisted of hollow-stem auger borings, cone penetration testing with seismic wave velocity measurements and pore pressure dissipation testing, piezometer installation and monitoring, and a laboratory program including strength and index testing. Data collected from the geotechnical investigation, available design drawings, construction records, inspection reports, previous engineering investigations, and other pertinent historic documents were utilized to perform the safety factor assessment and geotechnical analyses.

The East Ash Pond embankment is generally medium stiff to stiff and overlies predominantly alluvial foundation materials. The alluvial foundation consists of soft to stiff clay overlying medium dense to dense sand. A zone of sluiced flyash that existed before the embankment dike was constructed was encountered below the compacted embankment in the southeast corner. The zone of sluiced flyash was modified by the installation of Deep Mixing Method (DMM) ground improvement technology using the wet soil mixing method. Explorations were terminated in the soil overburden and were not extended to bedrock. The phreatic surface is typically at or slightly above the embankment/foundation interface.

Six (6) representative cross sections were analyzed using limit equilibrium slope stability analysis software to evaluate stability of the perimeter dike system and foundations. The cross sections were located to represent critical surface geometry, subsurface stratigraphy, and phreatic conditions across the CCR unit. Each cross section was evaluated for each of the loading conditions stipulated in §257.73(e)(1).

The Soils Susceptible to Liquefaction loading condition, §257.73(e)(1)(iv), was not evaluated because a liquefaction susceptibly evaluation did not find soils susceptible to liquefaction within the East Ash Pond dikes. As a result, this loading condition is not applicable to the East Ash Pond at the Joppa Power Station.

Results of the Initial Safety Factor Assessments, for the critical cross-section for each loading condition (i.e., the lowest calculated factor of safety out of the cross sections analyzed for each loading condition), are listed in Table 1.

Loading Conditions	§257.73(e)(1) Subsection	Minimum Factor of Safety	Calculated Factor of Safety
Maximum Storage Pool Loading	(i)	1.50	1.59
Maximum Surcharge Pool Loading	(ii)	1.40	1.57
Seismic	(iii)	1.00	1.01
Soils Susceptible to Liquefaction	(iv)	1.20	Not Applicable

Table 1 – Summary of Initial Safety Factor Assessments

Based on this evaluation, the East Ash Pond meets the requirements in §257.73(e)(1).

3 **Certification Statement**

CCR Unit: Electric Energy, Inc.; Joppa Power Station; East Ash Pond

I, Victor A. Modeer, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief that the information contained in this CCR Rule Report, and the underlying data in the operating record, has been prepared in accordance with the accepted practice of engineering. I certify, for the above-referenced CCR Unit, that the initial safety factor assessment dated October 14, 2016 meets the requirements of 40 CFR §257.73.

A. Modeer Vr

Printed Name

14/16 10

Date



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1001 Highlands Plaza Drive Wes Suite 300 St. Louis, MO 63110 1-314-429-0100 ATTACHMENT 9 – ADDENDUM TO THE CLOSURE PLAN (SEPTEMBER 2020)



40 C.F.R. § 257.102(B)(3): Closure Plan Addendum Joppa East Ash Pond September 29, 2020

ADDENDUM NO. 1 JOPPA EAST ASH POND CLOSURE PLAN

This Addendum No. 1 to the Closure Plan for Existing Coal Combustion Residuals (CCR) Impoundment for the Joppa East Ash Pond at the Joppa Power Station, Revision 0 - October 17, 2016 has been prepared to meet the requirements of Title 40 of the Code of Federal Regulations (40 C.F.R.) Section 257.103(f)(2)(v)(D) as a component of the demonstration that the Joppa East Ash Pond qualifies for a site-specific alternative deadline to initiate closure due to permanent cessation of a coal-fired boiler by a certain date.

The Joppa East Ash Pond will begin construction of closure by October 17, 2025 and cease receipt and placement of CCR and non-CCR wastestreams by no later than July 17, 2027 as indicated in the Joppa Power Station Alternative Closure Demonstration dated September 29, 2020. Closure will be completed by October 17, 2028 within the 5-year timeframe included in the Closure Schedule identified in the Joppa East Ash Pond Closure Plan in accordance with 40 C.F.R. § 257.102(f) (ii).

All other aspects of the Closure Plan remain unchanged.

CERTIFICATION

I, Eric J. Tlachac, a Qualified Professional Engineer in good standing in the State of Illinois, certify that the information in this addendum is accurate as of the date of my signature below. The content of this report is not to be used for other than its intended purpose and meaning, or for extrapolations beyond the interpretations contained herein.

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Eric J. Tlachac Qualified Professional Engineer 062-063091 Illinois Ramboll Americas Engineering Solutions, Inc., f/k/a O'Brien & Gere Engineers, Inc. Date: September 29, 2020







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